Clemons, Rutherford, & Associates, Inc.

2027 Thomasville Road Tallahassee, Florida 32308 Planners

LOCATION MAP

R



PROJECT INFORMATION

Architects

APPLICABLE CODES:

FLORIDA BUILDING CODE, BUILDING (FBC-B) FBC, CHAPTER 11, FLORIDA ACCESSIBILITY CODE FLORIDA BUILDING CODE, MECHANICAL (FBC-M) FLORIDA BUILDING CODE, FUEL GAS (FBC-FG) FLORIDA BUILDING CODE, PLUMBING (FBC-P) FLORIDA BUILDING CODE, EXISTING BUILDING (Repair) NFPA 101 LIFE SAFETY CODE FLORIDA FIRE PREVENTION CODE NATIONAL ELECTRICAL CODE (NEC)

6th EDITION 6th EDITION 6th EDITION 6th EDITION 6th EDITION 6th EDITION 2015 EDITION 6th EDITION 2017 EDITION

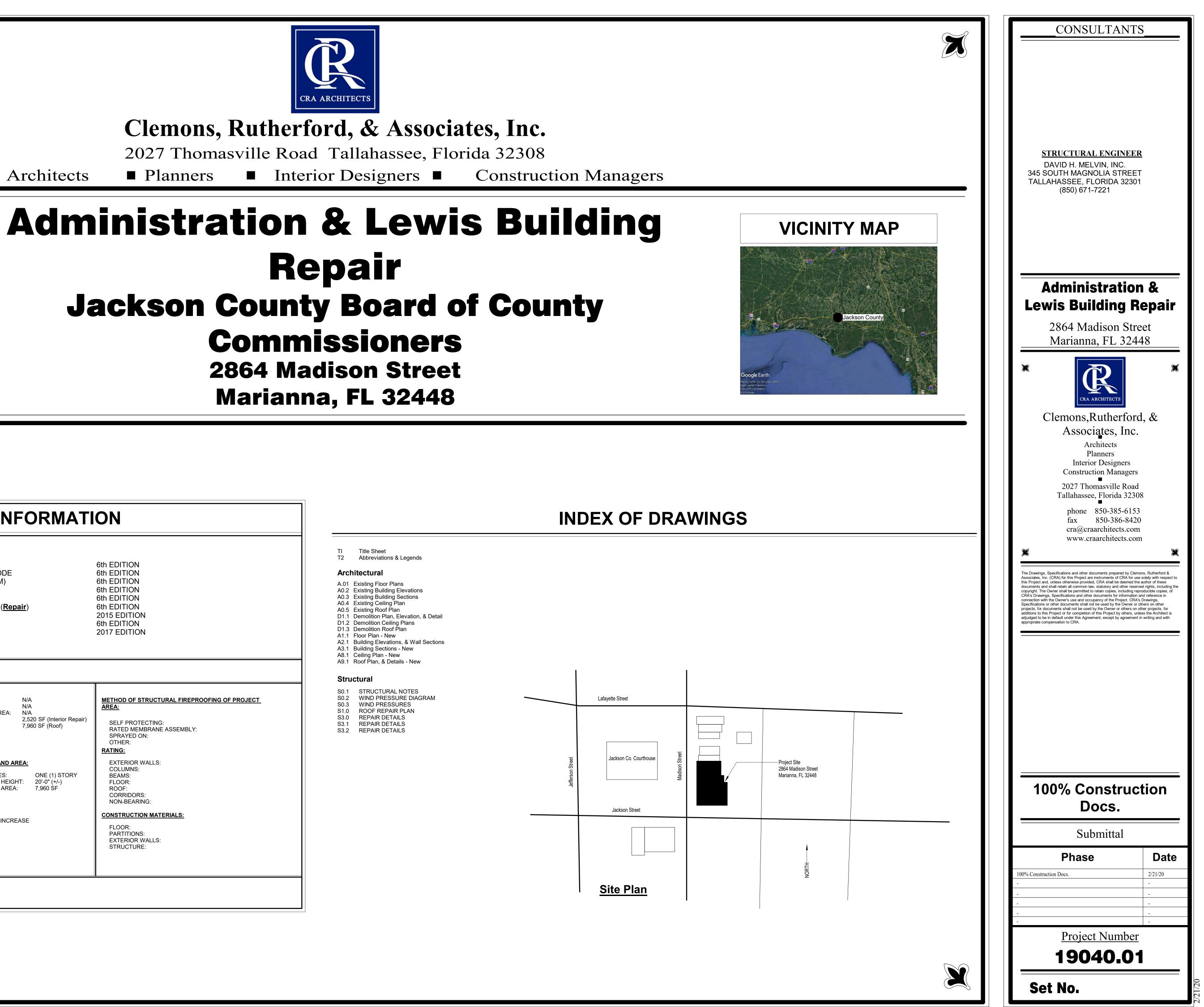
CONSTRUCTION TYPE:

PRINCIPAL OCCUPANCY:INSTITUTIONAL I-2:N/AAMBULATORY SURGICAL CENTER:N/ABUSINESS:YESOTHER:N/AACCESSORY OCCUPANCIES:N/AINSTITUTIONAL:N/AASSEMBLY:N/ASTORAGE:N/AOTHER:N/A	CON. NO. N/A EXPIRATION DATE: N/A CON. APPROVED AREA: N/A PROJECT AREA: 2,520 SF (Interior Repair) 7,960 SF (Roof) BUILDING HEIGHT AND AREA: NUMBER OF STORIES: ONE (1) STORY OVERALL BUILDING HEIGHT: 20'-0" (+/-) OVERALL BUILDING AREA: 7,960 SF PERCENT OF AREA INCREASE TAKEN:	METHOD OF STRUCTURAL FIREPR AREA: SELF PROTECTING: RATED MEMBRANE ASSEMBLY: SPRAYED ON: OTHER: RATING: EXTERIOR WALLS: COLUMNS: BEAMS: FLOOR: ROOF: CORRIDORS: NON-BEARING: CONSTRUCTION MATERIALS: FLOOR: PARTITIONS: EXTERIOR WALLS: STRUCTURE:

<u>NOTES:</u>



Repair **2864 Madison Street**



& Ճ_	And Angle	C.I. C.I.P.	Cast Iron Concrete In Place, Cast In Place Concrete	E.P. EPY.	Electrical Pane Epoxy Paint
@	At		Cast Iron Pipe	Eq., Eql.	Equal
<u>କ</u> [Center Line	C.J.	Control Joint	Equip.	Equipment
L	Channel	CL.	Closet	Equiv.	Equivalent
0	Diameter Round	C.L.	Center Line	E.R.S.P.	Elliptical Rein
	Degrees	CL. GL.	Clear Glass	E.S.	Exposed Struc
С	Degrees Celsius	Clg.	Ceiling	E.W.	Each Way
F	Degrees Fahrenheit	Clo.	Closet	E.W.C	Electric Water
•	Feet				Exhaust
>	Greater Than	Clr.	Clear, Clearance	Exh.	
••	Inches	СМ	Centimeter	Exist.	Existing
<	Less Than	C.M.U.	Concrete Masonry Unit	Exp.	Exposed, Exp
'	Minutes (Plane Angle)	C.N.L.	Conductive Neoprene Latex	Ext.	Exterior, Extru
1	Per	Cntr	Counter	F.	Fahrenheit
%	Percent	C.O.	Cased Opening, Clean Out	F.A.C.	Fire Apparatus
/0		Col.	Column	F.A.U.	Fresh Air Intak
<u>-</u> ±	Perpendicular	Com.	Common		
	Plus or Minus	Const.	Construction	FOB	Face of Brick
#	Pounds / Number	Cont.	Continuous	F.C.	Furring Chann
ዋ	Property Line / Plate	Coord.	Coordinate	F.C.U.	Fan Coil Unit
"	Seconds (Plane Angle)	Corr.	Corridor, Corrugated	F.D.	Floor Drain
	Square Feet	C.P.	Concrete Plaster	Fdn	Foundation
A,Amp	Ampere			F.D.V.C.	Fire Departme
A.B.	Anchor Bolt	Cpt	Carpet	F.E.	Fire Extinguis
A.C.	Above Ceiling	СРМ	Critical Path Method	F.E.C	Fire Extinguis
Acoust.	Acoustic, Acoustical	CPS	Cycles Per Second	F.F.	Finish Floor
A/C	Air Condition	C.R.	Cold Rolled	F.F.E	Finish Floor E
AC	Alternating Current	C.S.	Cup Sink, Clinical Sink, Countersunk	F.G.	Fixed Glass, F
	•	C.S.F	Hundred Square Feet	F.H.	
A.C.	Asbestos Cement Pipe	C.S.W.	Center Shaft Wall		Flat Head, Flue
A.C.U.	Air Conditioning Unit	C.T.	Ceramic Tile	F.H.C	Fire Hose Cab
A.C.T.	Acoustic Ceiling Tile	C.T.B.	Ceramic Tile Base	F.H.&E.C.	
A.D.	Access Door, Area Drain	C.T.P.	Ceramic Tile Panel	F.H.M.S	Flat Head Meta
Adj	Adjustable	Ctr.	Counter, Center	F.H.R.	Fire Hose Rac
A.D.O.	Automatic Door Operator		-	F.I.	Film Illuminate
A.F.F.	Above Finish Floor	CTS	Copper Tube Size	Fig.	Figure
A.H.	Air Handler, Access Hatch	Ctsk.	Countersink	Fin.	Finish
Alt.	Alternate, Altitude	Cu	Cubic	FI., FIr.	Floor
Alum	Aluminum	C.V.H.	Conductive Vinyle Homogenous (Sheet)	Fluor.	Fluorescent
A.L.P.	Acoustical Lay-in Panel	C.W.	Cold Water	F.O.S.	Face of Studs
Anod.	Anodized	C.Y.	Cubic Yard	F.P.	Fire Partition,
					•
A.P.	Access Panel, Acoustical Panel	D.B.A.	Deformed Bar Anchor	F.R.	Fire Rated, Fir
App.	Approximate	Dbl.	Double	Fr.	Frame
Arch.	Architectural	Dept.	Department	F.R.P.	Fiber Reinforc
A.S.	As Shown	Det.	Detail	F.S.	Floor Sink, Fu
Asph.	Asphalt	D.F.	Drinking Fountain	F.S.R.S.	Flexible Sheet
А.Т.	Acoustic Tile	Dia.	Diameter	Ft.	Foot, Feet
AVG	Average	Diag.	Diagnal, Diagram	Ftg.	Footing
		Dim.	Dimension	Furr.	Furring
В.	Bins	D.I.P.	Ductile Iron Pipe	Fut.	Future
B.B.	Bullentin Board	Disp.	Dispenser	F.W.P.	Full Weight Pa
B.C.	Book Case	Div.	Division, Divided	F.W.P.	Full Weight Pa
BD	Board	D.L.	Dead Load		
BE.	Bench	D.L. Dn.	Down	G	Gas
BF	Board Feet			Ga.	Gauge
B.G.	Bumper Guard	DP	Damproofing	Galv.	Galvanized
Bitum.	Bituminous	Dr.	Door	G.B.	Grab Bar
Bldg.	Building	Drwr.	Drawer	G.C.	General Contr
Blk(G)	Block(ing)	D.S.	Downspout	G.D.	Gypsum Deck
BM.	Beam, Bench Mark	D.B.S.	Double Strength "B" Quality Glass	Gen.	Generator
Btm	Bottom	DTL	Detail	Genl.	General
		D.W.	Dumbwaiter, Dishwasher		
Brg., Brng	•	Dwg	Drawing	GI	Glass, Glazing
BTU	British Thermal Units	Dwg's	Drawings	G.M.	Galvanized Me
B.U.	Built Up	DWV	Drain Waste Vent	G.M.L	Galvanized Me
B.U.R.	Built-Up Roofing			G.M.S	Galvanized Me
Civ.	Civil	E.	East	G.M.U	Glazed Mason
C/C	Center to Center	Ea.	Each	GPH	Gallons Per H
	Contractor Furnished /	E.B.	Expansion Bolt	GPM	Gallons Per M
C.F.C.I.	Contractor Furnished / Contractor Installed	E.C.	Exposed Concrete	Gr.	Grade
o		E.C.B.	Exposed Concrete Block	G.R.C	Glass Reinfor
CCF	Hundred Cubic Feet	EDP	Electronic Date Processing	G.S.	Gypsum Shea
CCTV	Closed Circuit Television	E.F.	Exhaust Fan	G.S.P	Galvanized St
Cab.	Cabinet	EIFS		G.W.B.	Gypsum Wallb
Conc.	Concrete		Exterior Insulation Finish System		
Cem.	Cement	E.J.	Expansion Joint	Gyp.	Gypsum
Cer.	Ceramic	Elec.	Electric(al)	H., Ht	Height
C.F.	Cubic Foot	Elev.	Elevator	H.B	Hose Bibb
C.FL.	Conductive Floor	EI.	Elevation	Hbd	Hardboard
CFM	Cubic Foot Per Minute	Emerg.	Emergency	H.C.	Hollow Core
C.G.	Corner Guard, Center of Gravity	J			
0.0.	Comer Guard, Center Of Glavity				

ABBREVIATIONS (ARCHITECTURAL)

Headed Concrete Anchor

High Point, Horse Power, Heat Pump

Handicapped

Hollow Metal

Horizontal

Hdwr., Hw. Hardware

H.C.A

H/C

Н.М.

Horiz.

H.P.

Mfd., Mfg Manufactured, Manufacture

trical Panel xy Paint al
ipment
ivalent
tical Reinforced Concrete Pipe
osed Structure
n Way
tric Water Cooler
aust
ting
osed, Expansion
rior, Extruded
renheit
Apparatus Closet
h Air Intake
e of Brick
ing Channel
Coil Unit
r Drain
ndation
Department Valve Cabinet
Extinguisher
Extinguisher Cabinet
sh Floor
sh Floor Elevation
d Glass, Floor Grate
Head, Flush Head, Fire Hydrant
Hose Cabinet
Hose & Extinguisher Cabinet
Head Metal Screw
Hose Rack
Illuminator
re
sh
r
rescent
e of Studs
Partition, Fire Protection
Rated, Fire Riser
ne
-
r Reinforced Polyester
r Sink, Full Size
ible Sheet Roofing System
, Feet
ling
ing
•
Weight Partition
Weight Partition
ae
ge
vanized
o Bar
eral Contractor
sum Deck
erator
eral
s, Glazing
vanized Metal
vanized Metal Lath
anized Metal Studs
ed Masonry Unit
ons Per Hour
ons Per Minute
de
s Reinforced Concrete
sum Sheathing
vanized Steel Pipe
sum Wallboard
sum
F.4
ht
e Bibb
lboard

Н.Р.	High Point, Horse Power, Heat Pump	Min
H.R.	Hand Rail	Mir.
Hr.	Hour	Mis
H.R.S	Health & Rehabilitative Services	Mld
H.S.	Heat Strengthened	M.L
Htg. H.V.	Heating Heat Seamed Soild Vinyl	M.N M.O
H.V.A.C	Heating, Ventilation, & Air Conditioning	Mod
H.W.	Hot Water	M.O
Hz.	Hertz (Cycles Per Second)	Mor
		M.P
I.D.	Inside Diameter	MP
ln.		M.R
Incl.	Include, Inclusive	M.S
Insul.	Insulation	M.S
Int., Intr Inv.	Interior Invert	M.T
I.P.	Isolation Panel	Mtd
I.P.S	Inside Pipe Size	M.W
IPS	Iron Pipe Size	Mt'l
I.T.	Isolation Transformer	Mul
I/O Cpt.	Indoor/Outdoor Carpet	Μv
•	·	Mw
Jan.	Janitor	(N)
J.B.	Junction Box Joist	Ν.
Jst. Jt	Joist	N.I.0
JL	JUST	N.L
K.E	Kitchen Equipment	No.
K.C.	Kneene's Cement	Non
Kit., K	Kitchen	N.R
K.O.	Knock Out	N.T.
K.P.	Kick Plate	0
K.S.	Knee Space	Obs
L	Ladder, Length, Landscaping	Oa.
Lab.	Laboratory	O.F
Lam.	Laminate	0.C
Lat.	Latitude	O.D
Lav.	Lavatory	~ ~
Lbl.	Label	Off.
Lb, Lbs	Pounds	0.H
L.C.	Laundry Chute	0.0
L.F.	Linear Feet	Opg Opp
Lg L.H.	Long Left Hand	0.W
Lin.	Linen	Oz
Lkr.	Locker	
L.L	Lead-Lined, Live Load	PI
L.L.D	Lead Lined Door	P(f)
L.P	Light Pole, Low Point	Par.
L.P.D	Light Proof Door	Part
L.P.S	Light Proof Shade	P.C. P.C.
Loc.	Location	P.H
Louv.	Louver, Louvered	Pil.
Lt.	Light	Plas
Ltg.	Lighting	Plas
Ltwt.	Lightweight	P.L.
М.	Thousands	Plyv
Mas.	Masonry	Pnl.
Mat.	Material	Pol.
Max.	Maximum	Por
М.В.	Machine Bolt, Marker Board	P.P.
M.B.M.	Metal Building, Manufacturer	P.P.
M.C.	Medicine Cabinet	Pr.
M.C.O	Metal Cased Opening	Pres
M.D.S	Metal Divider Strip	P.R
Mech.	Mechanic(al)	P.S
Med.	Medium	P.S.
Memb.	Memeber Metel	P.S.
Mtl. Mozz	Metal Mozzanino	P.T Pt
Mezz.	Mezzanine	۲1

Mfgr.	Manufacturer	Ptd
Mh. M/Hr.	Manhole Manhour(s)	P.V.0
MH	Megahertz	Pvm
Mil.	Millimeter	P.W.
Min.	Minimum, Minute	Q.T
Mir. Misc.	Mirror Miscellaneous	Qtr.
Mldg.	Moulding, Molding	(R)
M.L.W	Mean Low Water	R
M.N.I.C	Material Not in Contract	R.A.
M.O. Mod.	Masonry Opening Modified	Rad. R.B.
M.O.L	More or Less	R.C.
Mon.	Monument	R.C.I
M.P. MPH	Movable Partition Miles Per Hour	R.D.
M.R.	Moisture Resistant	Re. Rece
M.S.	Metal Stud, Metal Stair, Machine Screw	Recp
M.S.L	Mean Sea Level	Ref.
M.T. Mtd	Metal Threshold Mounted	Rfg. Reg.
M.W.R	Masonry Wall Reinforcing	Rein
Mt'l	Material	Req'
Mull.	Mullion	Res.
M∨ Mw	Megavolt Megawatt	Rev. R.H.
	•	IX.11.
(N) N.	New North	Rk.
N.I.C	Not In Contract	R.L Rm.
N.L	Neoprene Latex	R.0
No. Nom.	Number Nominal	R.O.
N.R.(C)	Noise Reduction (Coefficient)	R.P
N.T.S	Not to Scale	RPM R.R.
0	Diameter, Round	R.S
Obs.	Obscure	R.S.I
Oa.	Overall	Sec.
O.F.C.I O.C. (H/V)	Owner Furnished / Contractor Installed On Center (Horizontally / Vertically)	S&R
0.D.	Outside Dimension, Overall Dimension	S. S.A.I
	Outside Diameter	S.A.I San.
Off. O.H.	Office	S.A.
0.п. 0.0	Opposite Hand, Oval Head, Over Head Owner Furnished / Owner Installed	S.B
Opg.	Opening	S.B.(S.C
Opp.	Opposite	Sche
O.W Oz	Observation Window Ounce	Scrn
		S.C.I
PI P(f)	Plate, Property Line Plumbing (Fixture)	S.C.I S.C.\
Par.	Parallel	S.D
Part.	Partition(s)	Seal
P.C.C P.C.F	Precast Concrete Pounds Per Cubic Foot	Sect Serv
Р.Н	Pan Head	S.F
Pil.	Pilaster	S.F.(
Plas.	Plaster, Plastic	S.F.1
Plas. Lam. P.L.F	Plastic Laminate Pounds Per Lineal Foot	Sh., Shth
Plywd.	Plywood	Sim.
Pnl.	Panel	S.L
Pol. Port.	Polished Portable	S.M. S.M.
Port. P.P.	Power Pole	Sp.
P.P.G	Polished Plate Glass	S.P.I
Pr.	Pair	Spec
Prestr. P.R.V	Prestressed Power Roof Ventilator	Sq. Sq.F
P.R.V P.S	Power Roof Ventilator Pipe Space	Sq.I Sq.H
P.S.F	Pounds Per Square Foot	Sq.Ir
P.S.I	Pounds Per Square Inches	Sq.Y
P.T Pt	Pressure Treated, Pneumatic Tube Paint. Point	S.S S/S
P.T.D	Paper Towel Dispenser	Sta.

Painted Polyvinyl Chloride, О. Polyvinyl Chloride Pipe Pavement nt. Pass Window Quarry Tile Quater **Relocated Item Riser, Radius** Return Air Radius Rubber Base **Runner Channel** Reinforced Concrete Pipe Ρ Roof Drain **Relative To** Receptacle ep. Reception Reference Refrigerator **Reglet, Register** Reinforcing(ed) nf. ı'c Required Resilient Reverse, Revised (sion) Round Head, Robe Hook, Right Hand, Roof Hatch Rack Roof Ladder, Rain Leader Room **Rough Opening** W.(Right of Way **Retractable Partition Revolutions Per Minutes** Rest Room **Roof Scuttle** S.D Recessed Soap Dish Second Self & Rod South , Similar, Structural Sound Attenuation Blanket .В Sanitary **.**T Suspended Acoustical Tile Shower Bench Standard Building Code S.C Special Coating, Solid Core ned. Schedule Screen Spinal Cord Injury Shower Curtain Rod .R :W. Solid Core Wood Soap Dish, Storm Drain Sealant Section Service ν. Square Feet **.C.A** Square Feet Contract Area .T.U Structural Fcing Tile Unit Shl. Shelving, Shelf Sheathing hg. Similar Ships Ladder Sheet Metal 1.S Sheet Metal Screw Space(s) Sound Proof Door D.' ec.(s) Specification(s) Square Ft. Square Feet Hd. Square Head Square Inch ln. Yd. Square Yard **Stainless Steel Sanitary Sewer** Station

Std.

Stl.

Stor.

Struct.

Susp.

S.W

Т.

Ta.

T.B

T.& B.

Tel.

Terr.

T&G

T.H

Thk.

Thru.

T.O.C

T.O.S

T.P.D

T.P.H

Treat.

Ts.

T.T

ΤV

Тур.

Uc.

Ur.

U.S

Util.

V.A

V.B.

V.C.T

Vert.

Vest.

V.H.

V.I.F

V.P

V.T.R

V.T.S

Vin.

V.W.C

V.W.E

Wa.

W.

W/

Wains.

W.B.

W.C.

W/C

Wd.

W.D

WDW

W.F

W.H

W.I

W.M

W.O

W/O

W.P

W.R

W.S

Wt.

Wtr.

W.W.F

W.W.M

Yd.

Yr.

W.C.O

U.N.O

Tr.

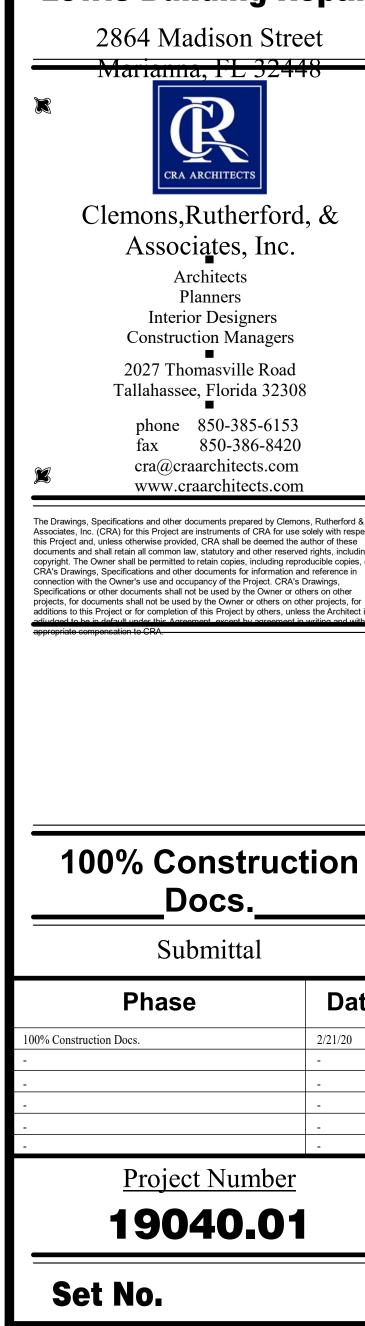
Toil.

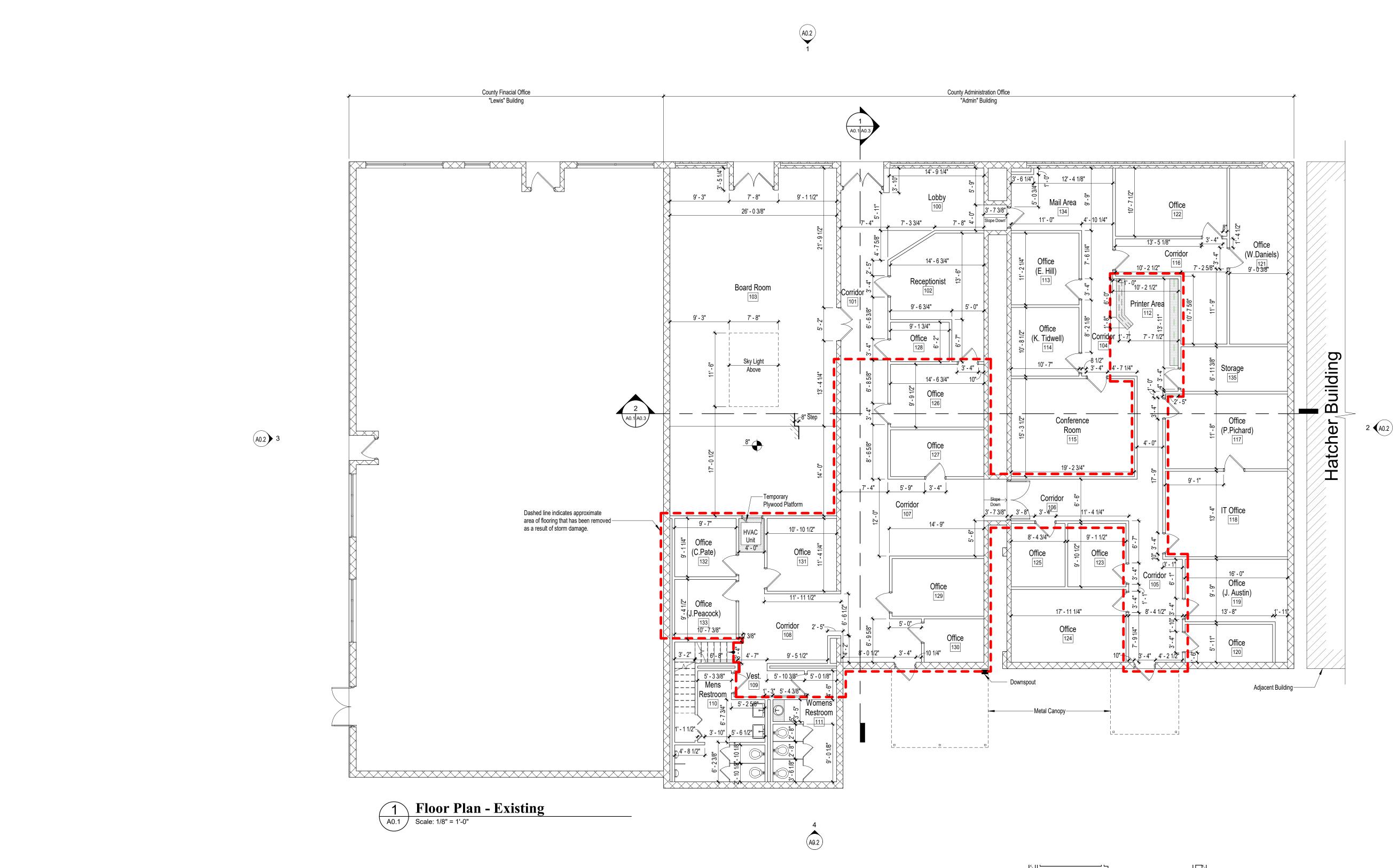
Thres.

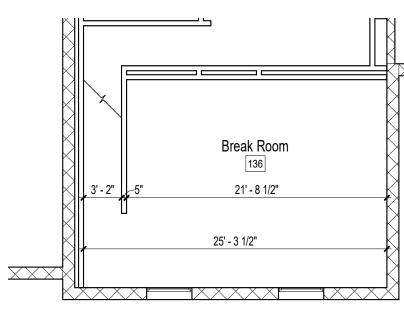
Temp.

Standard Steel Storage Structutral Suspend(ed) Storm Water Title Table Toggle Bolt, Towel Bar Top & Bottom Telephone Tempered, Temperature Terrazzo Tongue & Groove **Truss Head** Thick(ness) Threshold Through Top Of Concrete Toilet Top of Steel, Slab or Structure Toilet Paper Dispenser **Toilet Paper Holder** Tread Treatment Thinset Terrazzo Tile Resinous Matrix Television Typical Undercut **Unless Noted Otherwise** Urinal **Urinal Screen** Utility Volt Veterans Administration Vinyl Base Vinyl Composition Tile Vertical(ly) Vestibule Vinyl Homogenous Verify In Field **Veneer Plaster** Vent Throuh Roof Vinyl Transition Strip Vinyl Vinyl Wallcovering Verify With Equipment Water West, Width, Wall Fabric, Watt With Wainscot Wall Bumper Guard Water Closet Wheelchair Wood Cased Opening Wood Window Dimension Window Wide Flange Beam Water Heater Wrought Iron Water Meter Where Occurs Without Waterproof(ing) Waste Receptacle Wood Screw Weight Water Welded Wire Fabric Welded Wire Mesh Yard Year

Administration & Lewis Building Repai

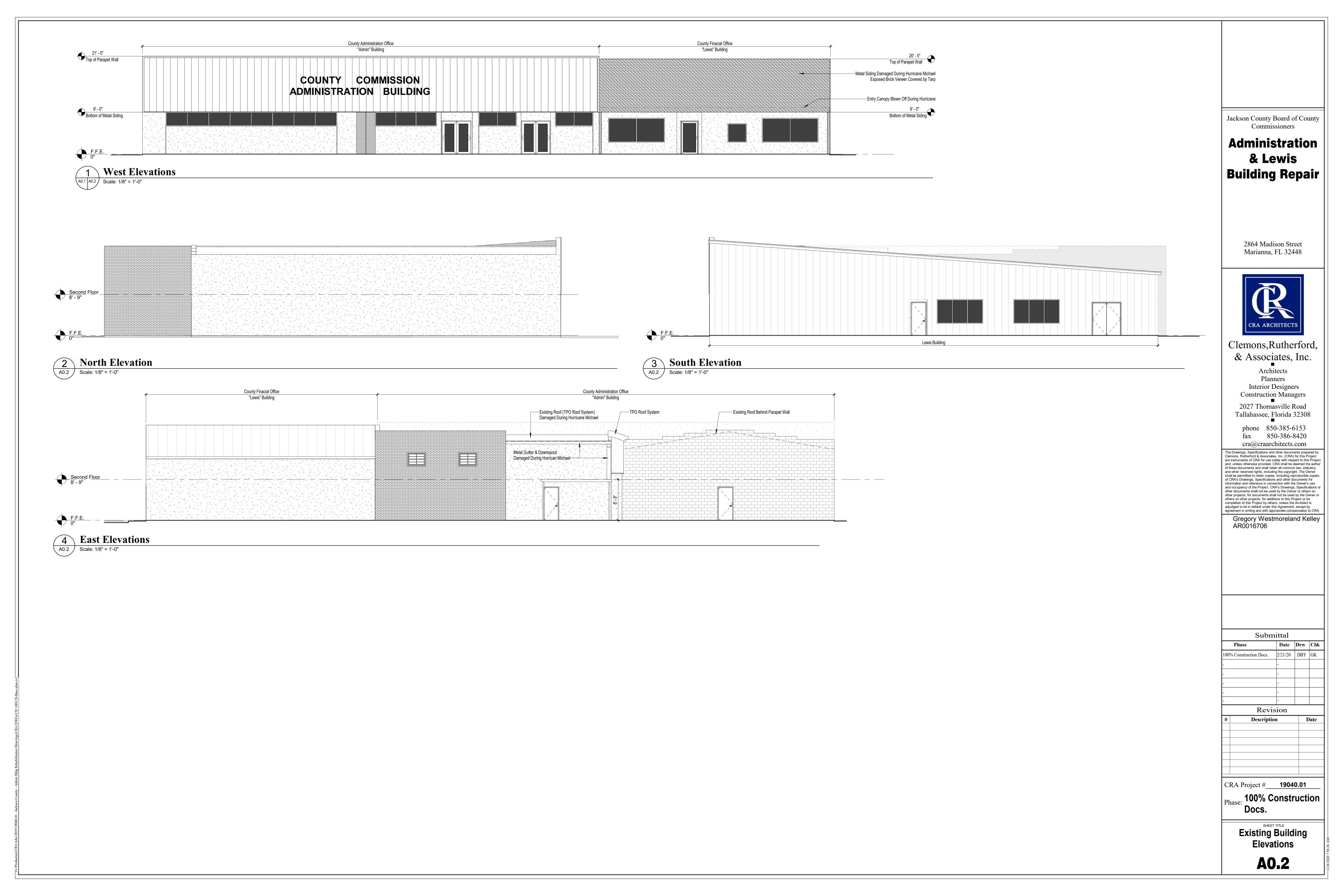


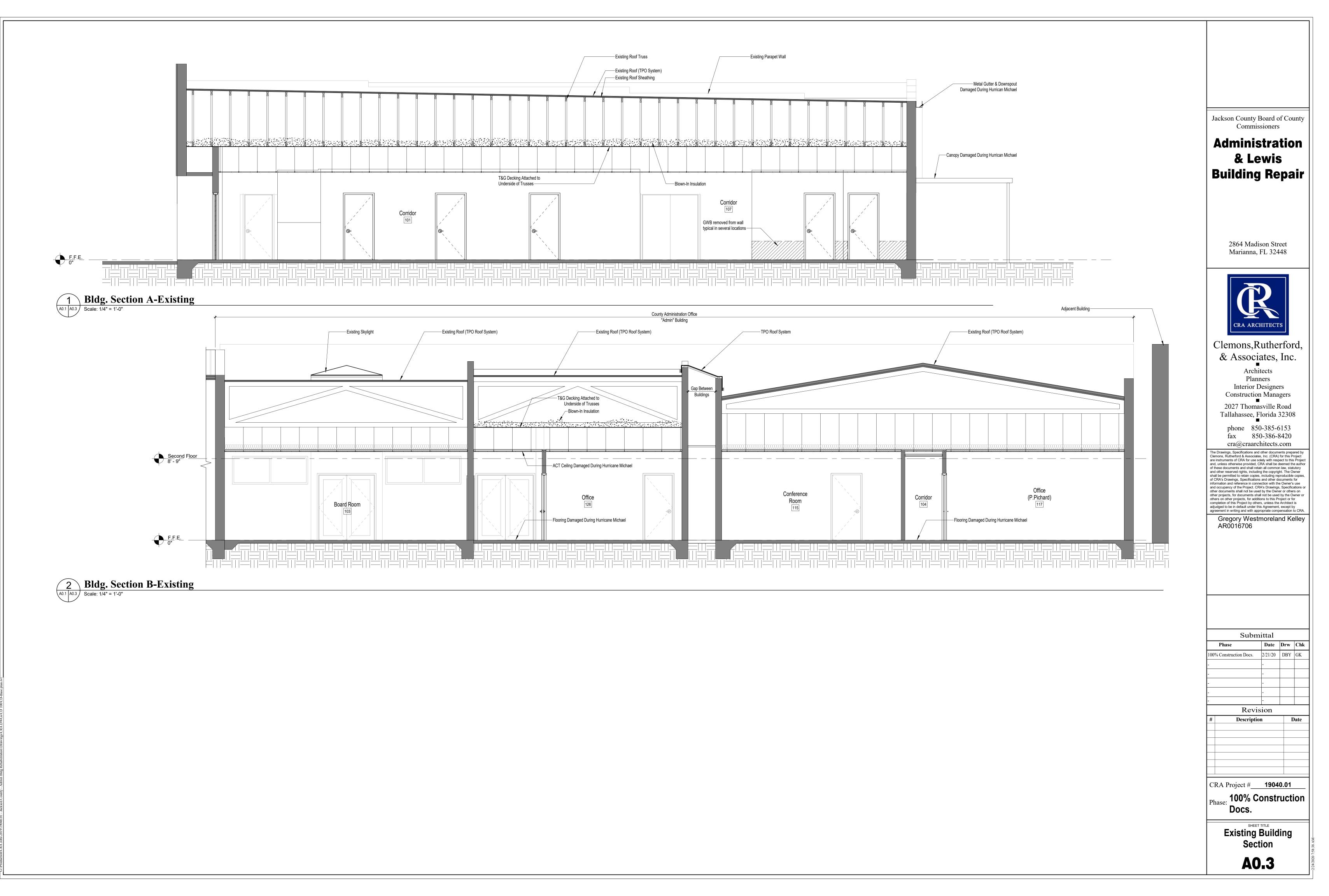




2 Second Floor - Existing A0.1 Scale: 1/8" = 1'-0"

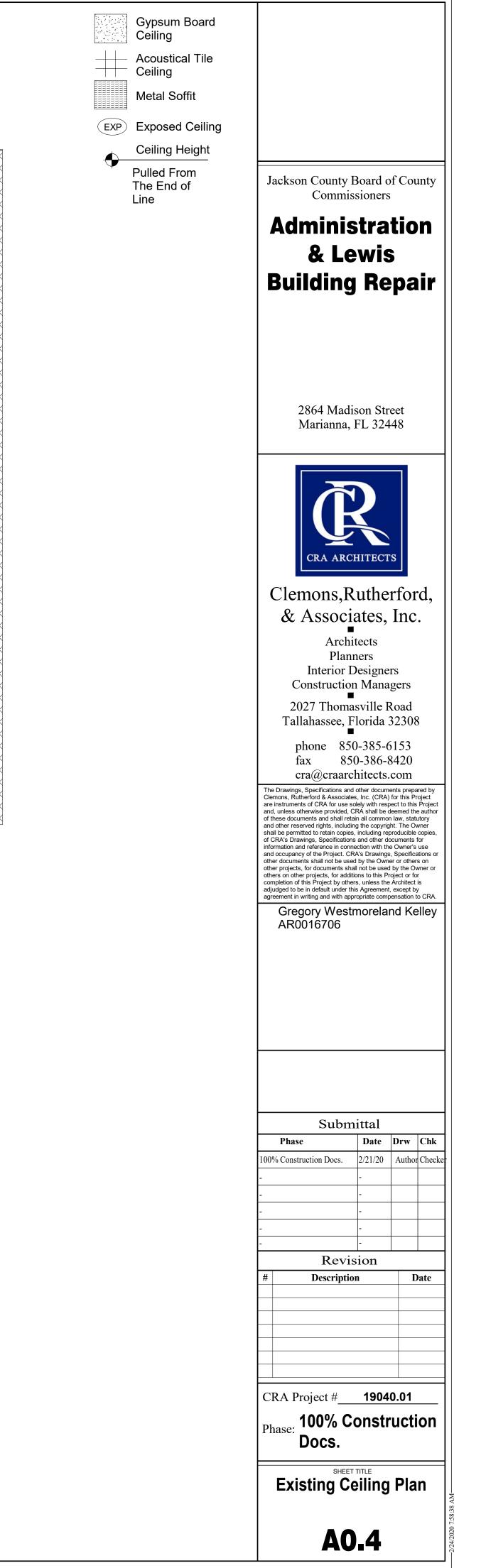


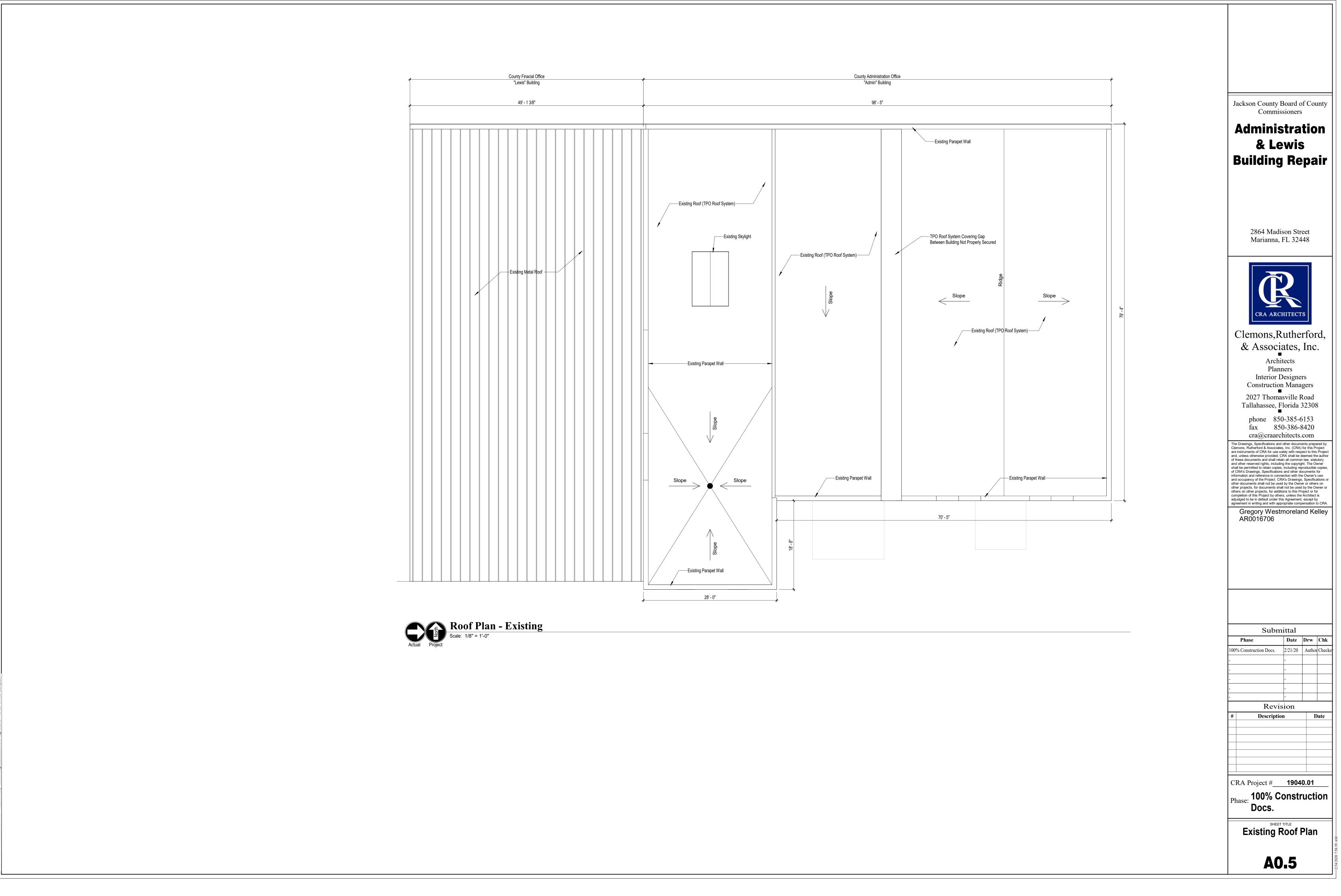














General Notes

- Coordinate all demolition and repair work with Owner sa as to not interrupt normal operation. Areas adjacent to construction are to remain occupied
- Consult with Owner to determine any salcageable items. These items shall be cleaned and delivered to a storage location as directed by owner. All other demolished material shall be delivered to a legal dump site.

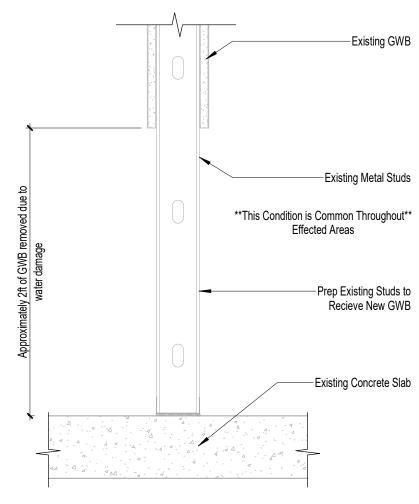
Hazardous Materials Notes

All hazardous items shall be taken to an approved disposal site.

- Demo existing roof down to existing structure and prep for new roof installation.

- (4) Demo temporary floor covering and prep for new floor finish installation
- Remove temporary plywood structure and temporary HVAC unit. Return HVAC unit
- Demo existing doors. All door hardware is to be saved and reused.
- Remove existing ceiling tiles and grid. Remove all light fixtures, and diffusers and store in secure location. Reinstall light fixtures and diffusers back to their original

- Remove existing tarp from face of building and prep to recieve new metal siding.





Jackson County Board of County Commissioners Administration & Lewis **Building Repair**

> 2864 Madison Street Marianna, FL 32448



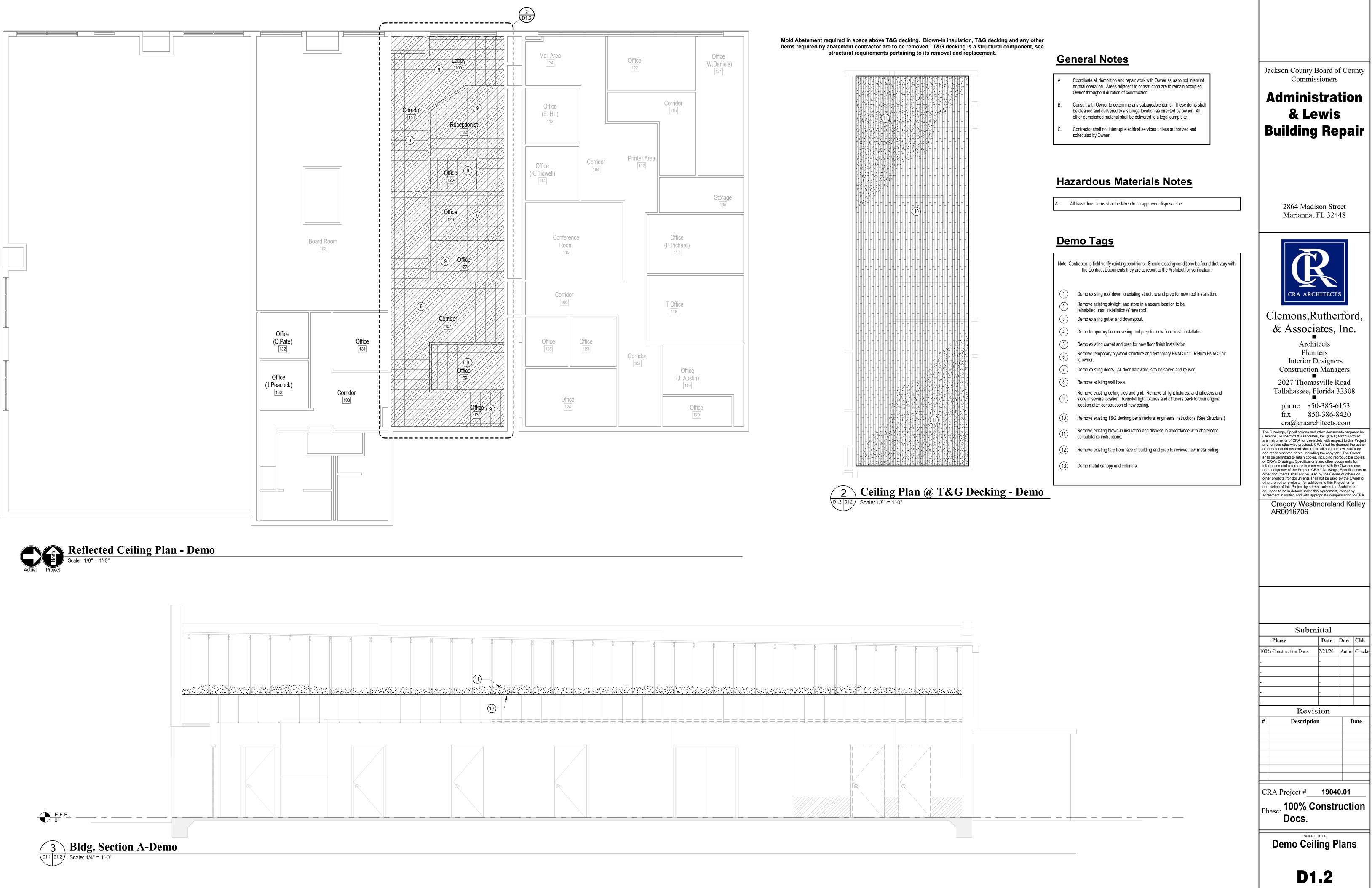
Clemons, Rutherford, & Associates, Inc. Architects Planners Interior Designers Construction Managers 2027 Thomasville Road Tallahassee, Florida 32308 phone 850-385-6153 850-386-8420 fax cra@craarchitects.com The Drawings, Specifications and other documents prepared by Clemons, Rutherford & Associates, Inc. (CRA) for this Project are instruments of CRA for use solely with respect to this Project and, unless otherwise provided, CRA shall be deemed the author and other reserved rights, including the copyright. The Owner shall be permitted to retain copies, including reproducible copies of CRA's Drawings, Specifications and other documents for information and reference in connection with the Owner's use and occupancy of the Project. CRA's Drawings, Specifications or and occupancy of the Project. CRA's Drawings, Specifications or other documents shall not be used by the Owner or others on other projects, for documents shall not be used by the Owner or others on other projects, for additions to this Project or for completion of this Project by others, unless the Architect is adjudged to be in default under this Agreement, except by agreement in writing and with appropriate compensation to CRA.

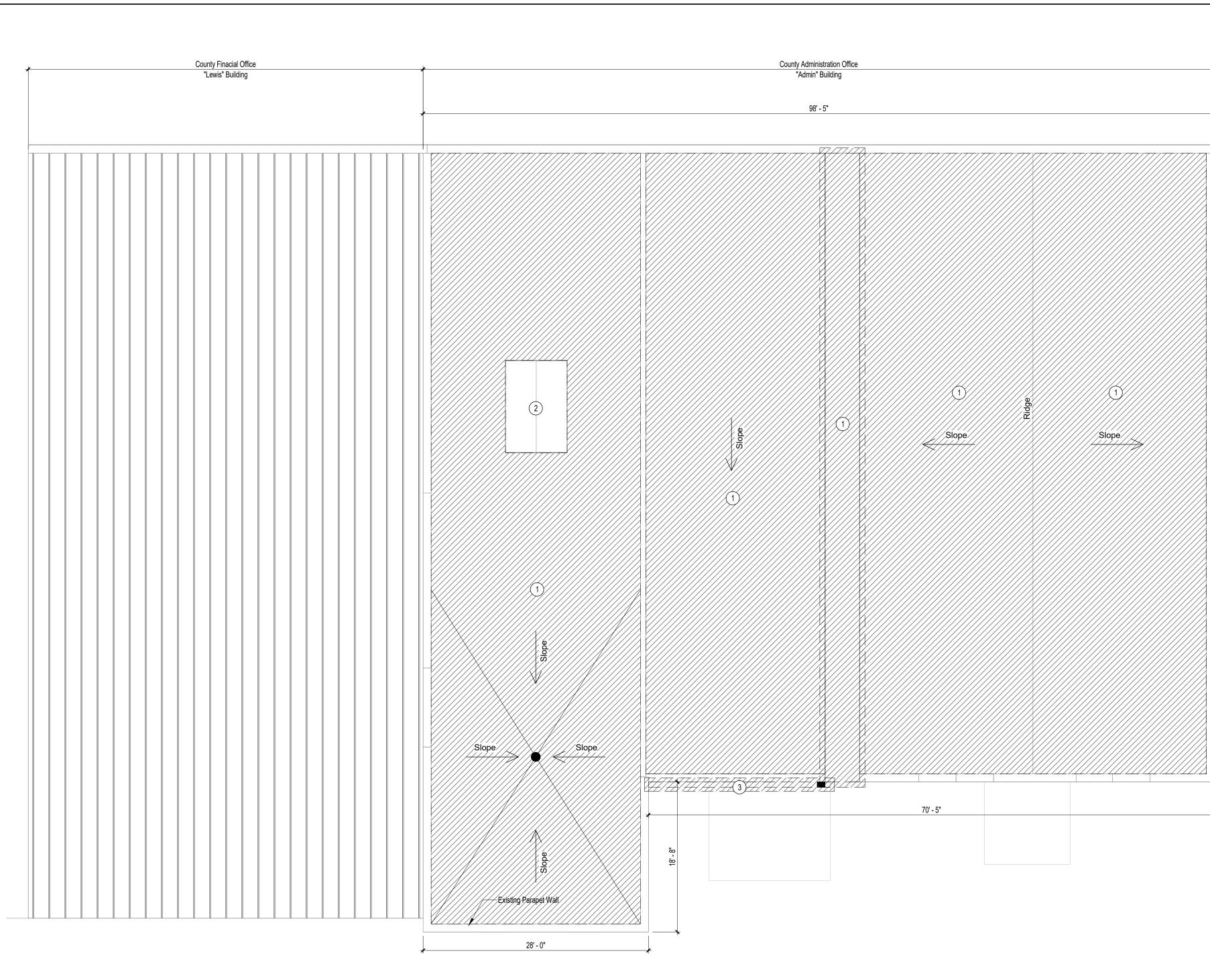
Gregory Westmoreland Kelley AR0016706

	Subn	nittal			
	Phase	Date	Drw	Chk	
100	% Construction Docs.	2/21/20	DBY	GK	
-		-			
-		-			
-		-			
-		-			
-		-			
	Revi	sion			
#	Descriptio	on		Date	
CRA Project # 19040.01					
Phase: 100% Construction Docs.					

SHEET TITLE Demolition Floor Plan, Elevation, &Detail

D1.1







Roof Plan - Demo Scale: 1/8" = 1'-0"

General Notes

- Coordinate all demolition and repair work with Owner sa as to not interrupt normal operation. Areas adjacent to construction are to remain occupied Owner throughout duration of construction.
- Consult with Owner to determine any salcageable items. These items shall be cleaned and delivered to a storage location as directed by owner. All other demolished material shall be delivered to a legal dump site.
- Contractor shall not interrupt electrical services unless authorized and scheduled by Owner.

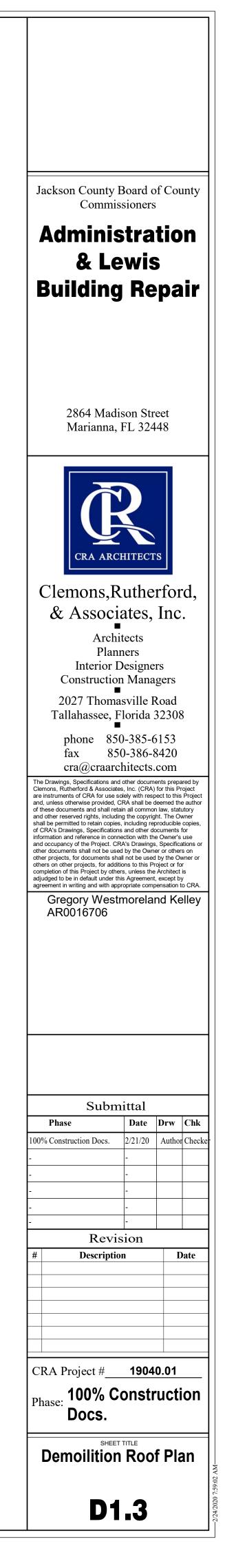
Hazardous Materials Notes

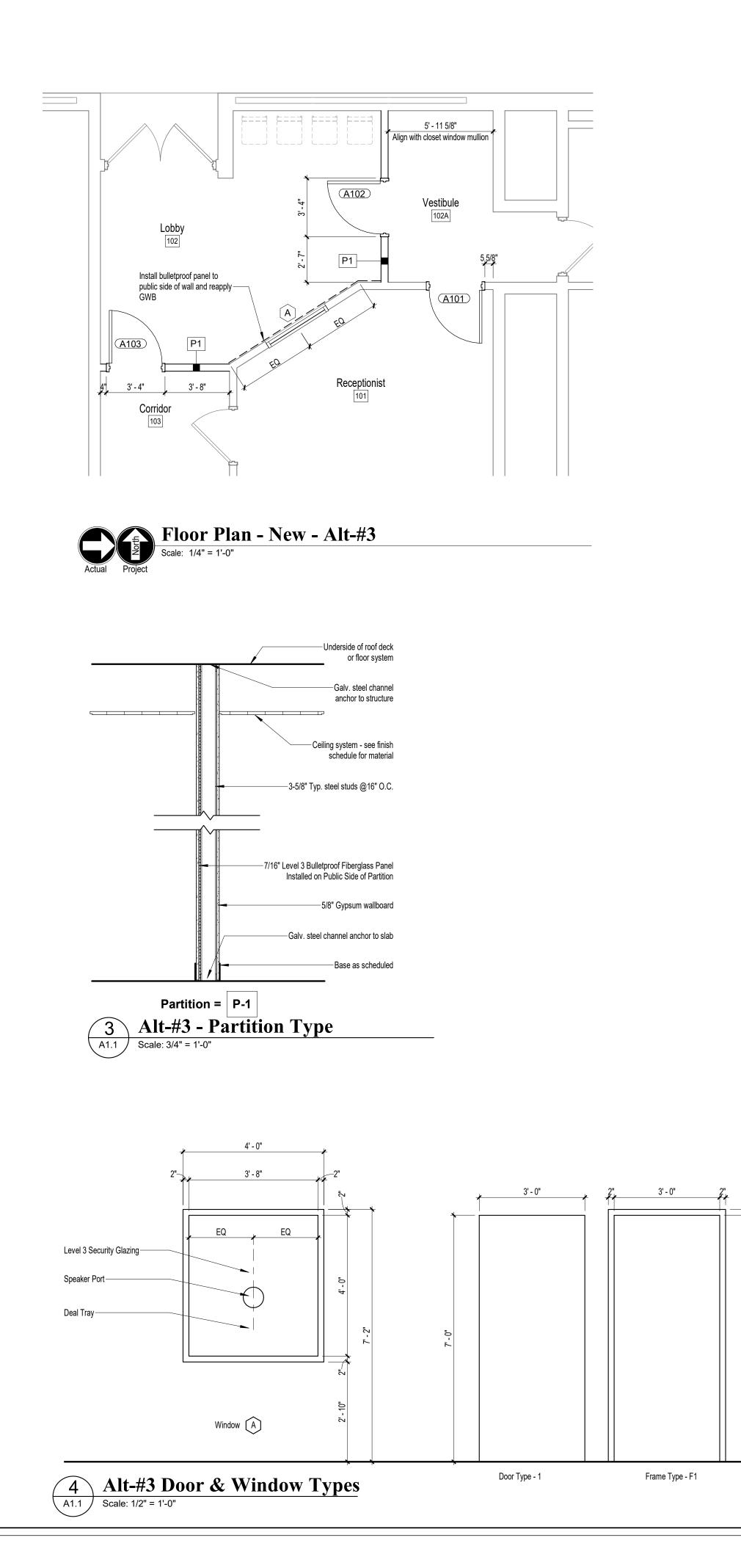
All hazardous items shall be taken to an approved disposal site.

<u>Demo Tags</u>

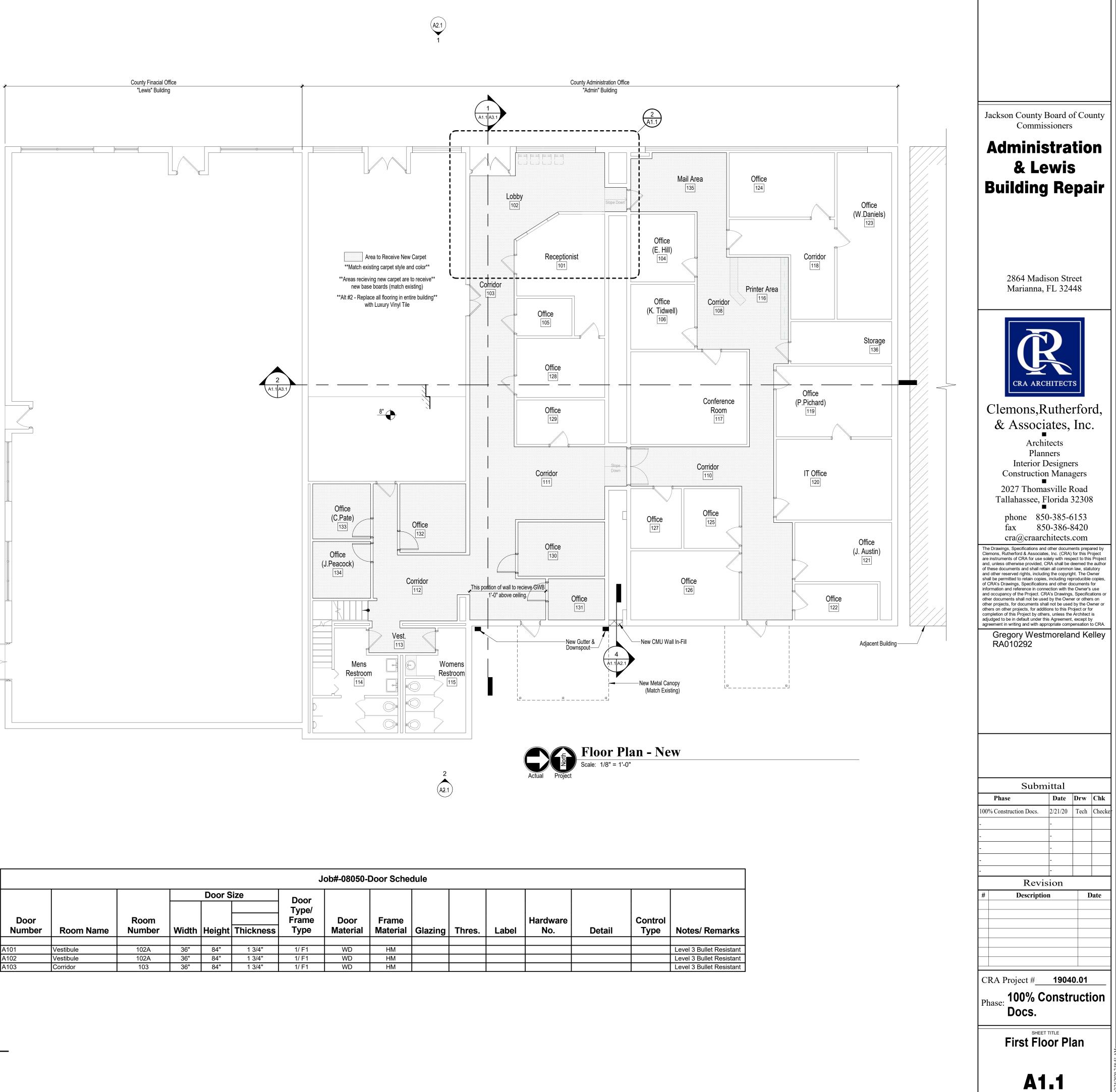
Note: Contractor to field verify existing conditions. Should existing conditions be found that vary with the Contract Documents they are to report to the Architect for verification.

- (1) Demo existing roof down to existing structure and prep for new roof installation.
- Remove existing skylight and store in a secure location to be reinstalled upon installation of new roof. 2
- 3 Demo existing gutter and downspout.
- 4 Demo temporary floor covering and prep for new floor finish installation
- 5 Demo existing carpet and prep for new floor finish installation 6 Remove temporary plywood structure and temporary HVAC unit. Return HVAC unit to owner.
- (7) Demo existing doors. All door hardware is to be saved and reused.
- 8 Remove existing wall base.
- Remove existing ceiling tiles and grid. Remove all light fixtures, and diffusers and store in secure location. Reinstall light fixtures and diffusers back to their original location after construction of new ceiling. 9
- (10) Remove existing T&G decking per structural engineers instructions (See Structural)
- Remove existing blown-in insulation and dispose in accordance with abatement (11) consulatants instructions.
- (12) Remove existing tarp from face of building and prep to recieve new metal siding.
- (13) Demo metal canopy and columns.

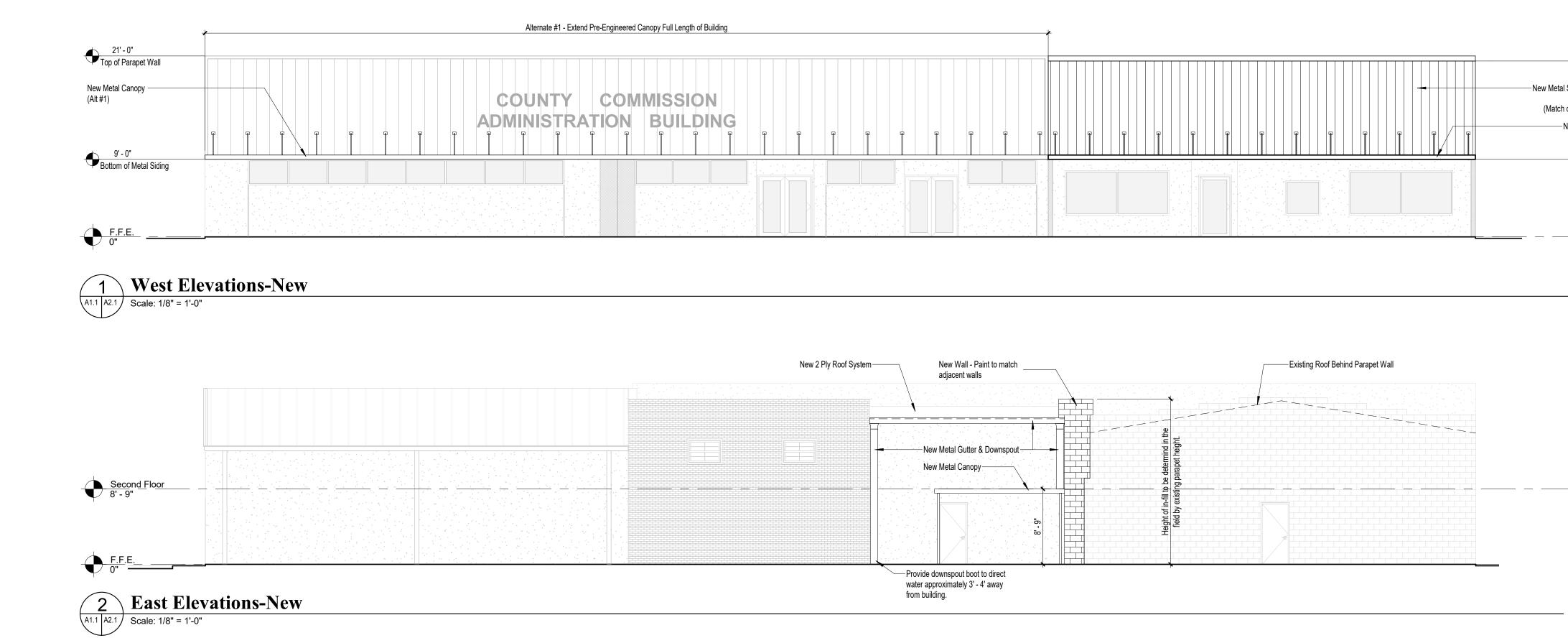




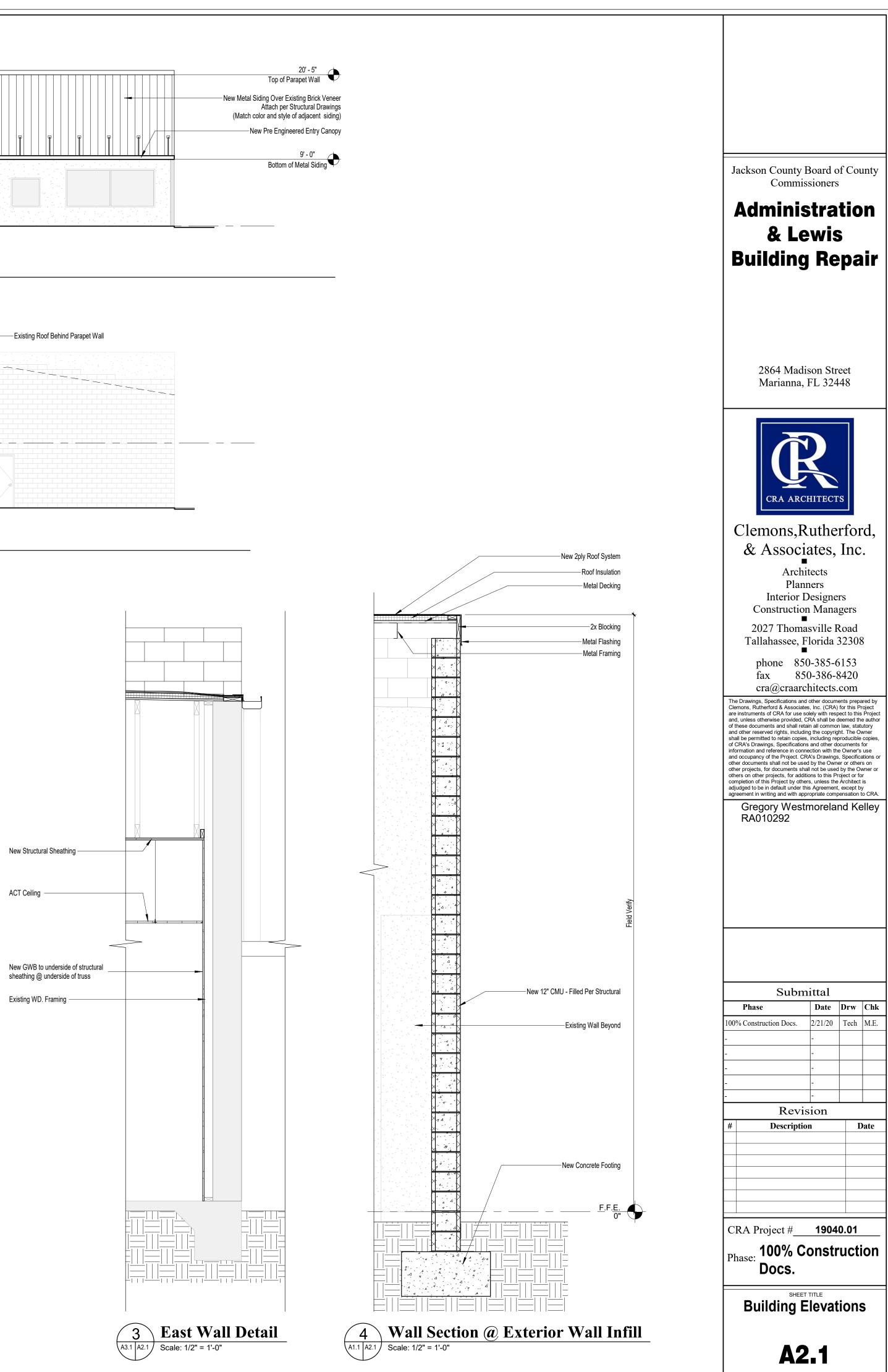


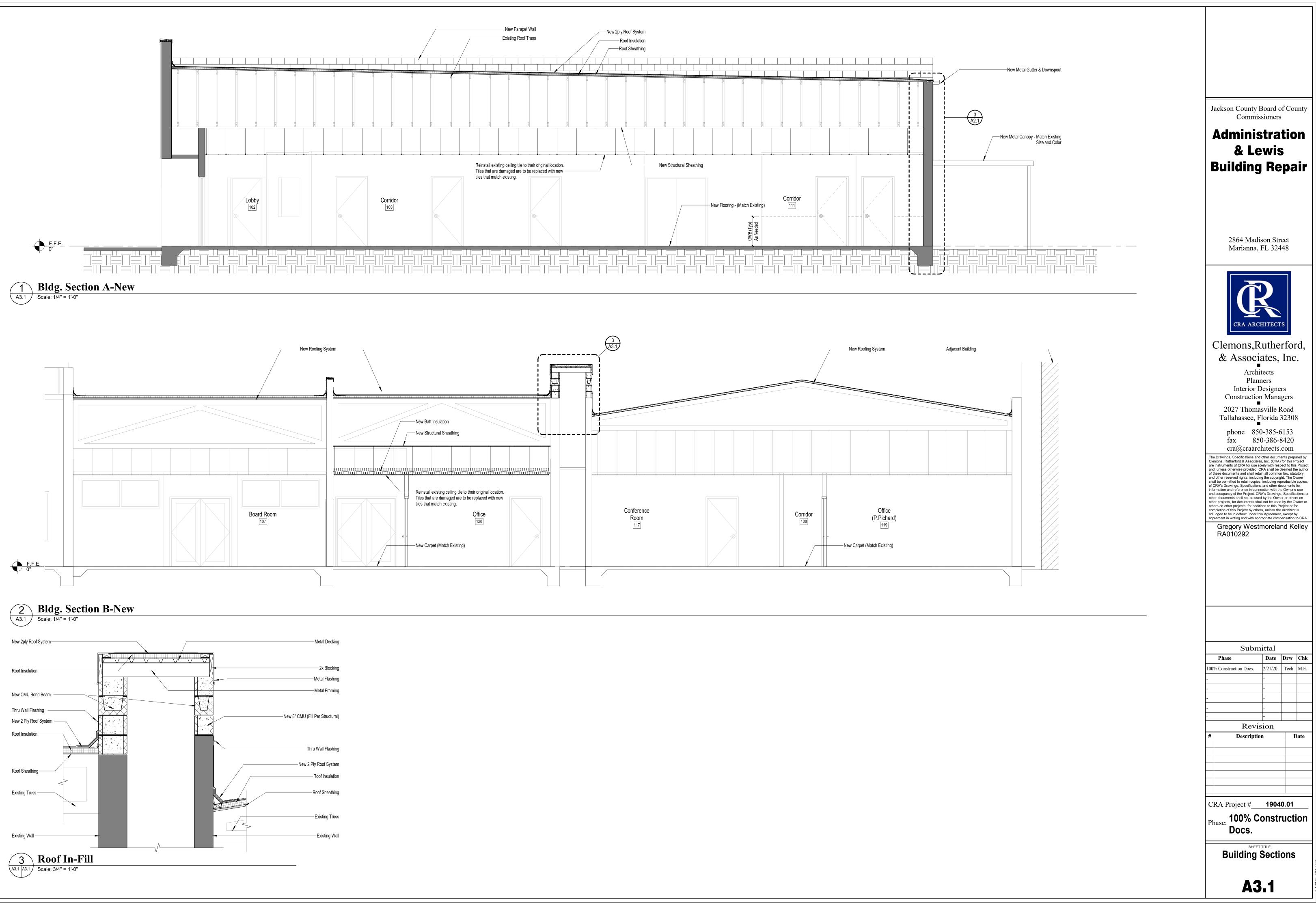


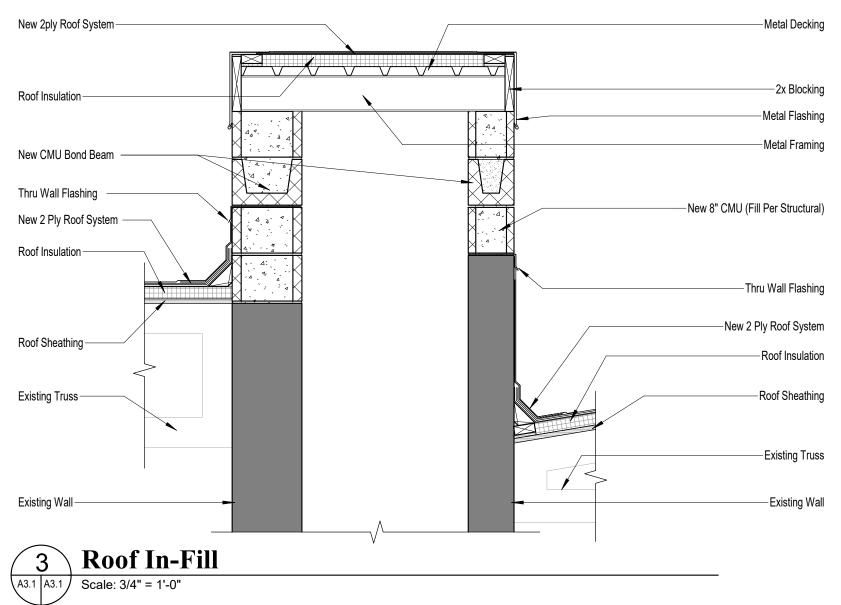
Job#-08050-Door Schedule													
			Door Size		Door								
Door Number	Room Name	Room Number	Width	Height	Thickness	Type/ Frame	Door Material	Frame Material	Glazing	Thres.	Label	Hardware No.	Deta
A101	Vestibule	102A	36"	84"	1 3/4"	1/ F1	WD	HM					
A102	Vestibule	102A	36"	84"	1 3/4"	1/ F1	WD	HM					
A103	Corridor	103	36"	84"	1 3/4"	1/ F1	WD	НМ					

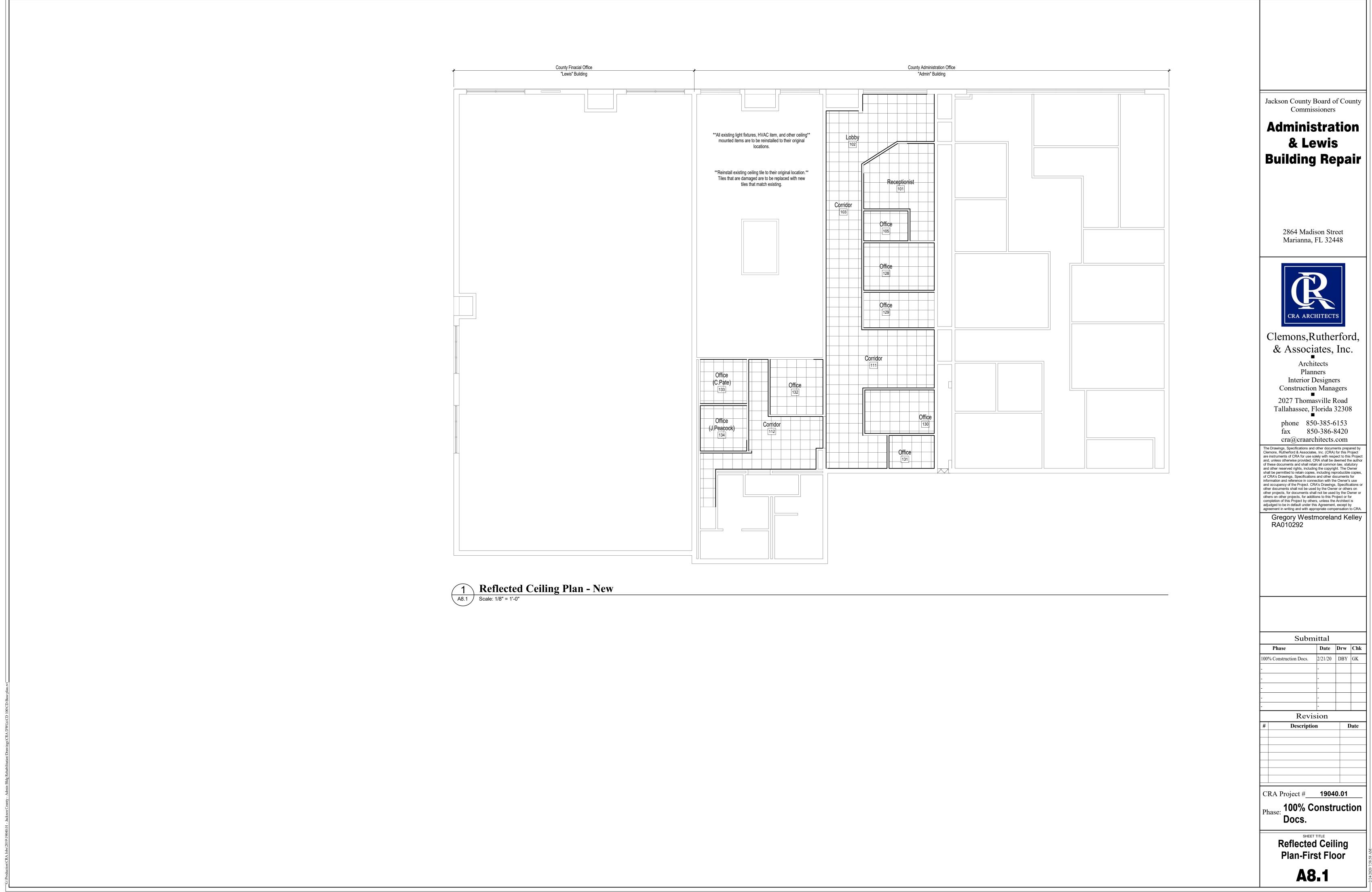


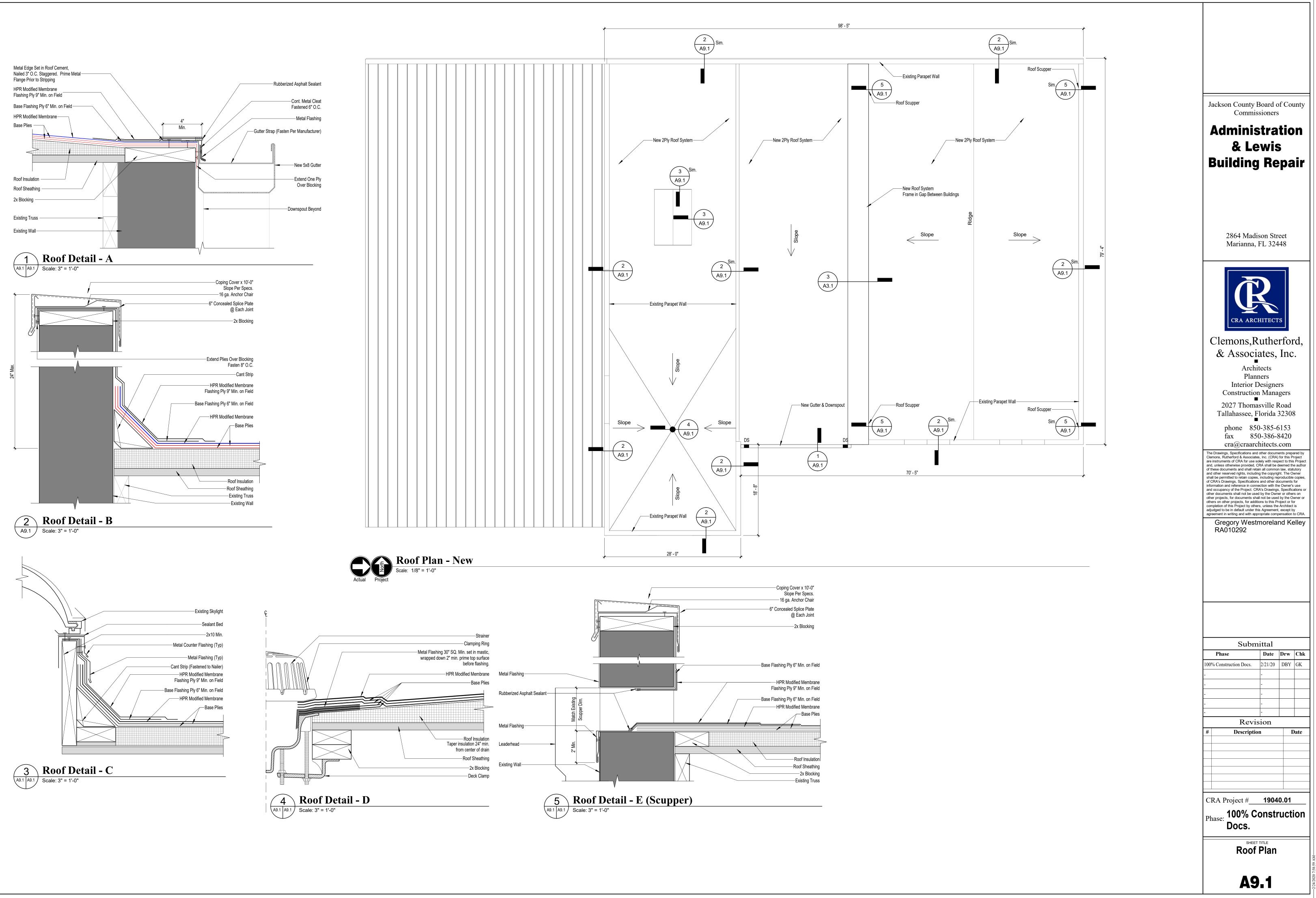
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		T New Metal Siding (Atta (Match color ar New Pre
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GENERAL NOTES

1. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS OF ALL OTHER DISCIPLINES AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, CHASES, HANGERS, INSERTS, ANCHORS, HOLES, AND OTHER ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.

2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS DURING THE WORK. THE ENGINEER WILL NOT ADVISE ON OR ISSUE DIRECTION AS TO SAFETY PRECAUTIONS AND PROGRAMS.

3. THE STRUCTURAL DRAWINGS HEREIN REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL ALL STRUCTURAL WORK AND CONNECTIONS HAVE BEEN COMPLETED. THE INVESTIGATION, DESIGN, SAFETY, ADEQUACY, AND INSPECTION OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

4. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR THE METHODS, TECHNIQUES, AND SEQUENCES OF PROCEDURES TO PERFORM THE WORK. THE SUPERVISION OF THE WORK IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR

5. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO APPROVAL BY THE ENGINEER.

6. ALL STRUCTURAL SYSTEMS WHICH ARE TO BE COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE, AND ERECTION IN ACCORDANCE WITH THE SUPPLIER'S INSTRUCTIONS AND REQUIREMENTS.

7. LOADING APPLIED TO THE STRUCTURE DURING THE PROCESS OF CONSTRUCTION SHALL NOT EXCEED THE SAFE LOAD-CARRYING CAPACITY OF THE STRUCTURAL MEMBERS. THE LIVE LOADING USED IN THE DESIGN OF THIS STRUCTURE ARE INDICATED IN THE "DESIGN CRITERIA NOTES". DO NOT APPLY ANY CONSTRUCTION LOADS UNTIL STRUCTURAL FRAMING IS CONNECTED TOGETHER AND UNTIL ALL TEMPORARY BRACING IS IN PLACE.

8. ALL ASTM AND OTHER REFERENCES ARE PER THE LATEST EDITIONS OF THESE STANDARDS, UNLESS OTHERWISE NOTED.

9. SHOP DRAWINGS AND OTHER ITEMS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE GENERAL CONTRACTOR BEFORE SUBMITTAL. THE ENGINEER'S REVIEW IS TO BE FOR CONFORMANCE WITH THE DESIGN CONCEPT AND GENERAL COMPLIANCE WITH THE RELEVANT CONTRACT DOCUMENTS. THE ENGINEER'S REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW, CHECK, AND COORDINATE THE SHOP DRAWINGS PRIOR TO SUBMISSION. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, DIMENSIONS, ETC.

10. ALL "STRUCTURAL SUBMITTALS" SHALL BE PREPARED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF FLORIDA. DRAWINGS PREPARED SOLELY AS A GUIDE FOR ERECTION, INSTALLATION, AND CATALOG INFORMATION WILL NOT REQUIRE AN ENGINEER'S SEAL; HOWEVER, THEY SHALL BEAR THE ENGINEER'S SIGNATURE AND AN INDICATION THAT HE OR SHE CHECKED THE WORK.

11. DRAWINGS INTRODUCING ENGINEERING INPUT AND CALCULATIONS SHALL BE SIGNED, SEALED, AND DATED BY THE ENGINEER PREPARING SUCH WORK.

DESIGN CRITERIA

1. THE INTENDED DESIGN STANDARDS AND/OR CRITERIA ARE AS FOLLOWS:

- FLORIDA BUILDING CODE 6TH EDITION (2017), BUILDING **GENERAL** COLD-FORMED STEEL 2012 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE ACI 318-14 CONCRETE BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530-13/ASCE 5-13/TMS-13) MASONRY NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION WITH 2015 NDS SUPPLEMENT WOOD
- 2. DESIGN SUPERIMPOSED GRAVITY DEAD LOADS USED IN THE DESIGN OF THIS STRUCTURE ARE AS FOLLOWS:

ACTUAL SELF-WEIGHT

3. DESIGN SUPERIMPOSED GRAVITY LIVE LOADS USED IN THE DESIGN OF THIS STRUCTURE ARE AS FOLLOWS:

UNIFORM

20 PSF

- **OCCUPANCY**
- ROOF
- 4. DESIGN LATERAL LIVE LOADS USED IN THE DESIGN OF THIS STRUCTURE ARE AS FOLLOWS:
- WIND LOADS PER ASCE 7-10 (3-SEC GUST)
- ULTIMATE WIND SPEED = 125 MPH
- RISK CATEGORY II EXPOSURE B

INTERNAL PRESSURE COEFFICIENT, GCpi = ±0.18 (LEWIS BUILDING [ENCLOSED]), ±0.55 (ADMINISTRATIVE BUILDING [PARTIALLY ENCLOSED])

5. THIS STRUCTURE HAS BEEN DESIGNED WITH "SAFETY FACTORS" IN ACCORDANCE WITH GENERALLY ACCEPTED PRINCIPLES OF STRUCTURAL ENGINEERING. THE FUNDAMENTAL NATURE OF THE "SAFETY FACTOR" IS TO COMPENSATE FOR UNCERTAINTIES IN THE INTENDED DESIGN. FABRICATION AND ERECTION OF STRUCTURAL BUILDING COMPONENTS. IT IS INTENDED THAT "SAFETY FACTORS" BE USED SO THAT THE LOAD CARRYING CAPACITY OF THE STRUCTURE DOES NOT FALL BELOW THE DESIGN LOAD AND THAT THE BUILDING WILL PERFORM UNDER DESIGN LOAD WITHOUT DISTRESS. WHILE THE USE OF "SAFETY FACTORS" IMPLIES SOME EXCESS CAPACITY BEYOND DESIGN LOAD, SUCH EXCESS CAPACITY CANNOT BE ADEQUATELY PREDICTED AND SHALL NOT BE RELIED UPON.

FOUNDATION NOTES

1. ALL FOOTINGS HAVE BEEN DESIGNED USING AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF. ALL FOUNDATION EXCAVATIONS SHALL BE EVALUATED BY THE GEOTECHNICAL ENGINEER/TESTING AGENCY PRIOR TO PLACING FOUNDATION CONCRETE.

2. AT FOOTING SUBGRADES, AT LEAST ONE TEST OF EACH SOIL STRATUM WILL BE PERFORMED FOR EACH ISOLATED FOOTING AND ONE TEST FOR EACH 75 LINEAR FEET OF CONTINUOUS WALL FOOTING PER LIFT TO VERIFY DESIGN **BEARING CAPACITIES.**

3. ALL FOUNDATION CONCRETE SHALL OBTAIN A 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI.

4. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE BUILDINGS". HOT WEATHER CONCRETING SHALL BE IN ACCORDANCE WITH ACI 305. COLD WEATHER CONCRETING SHALL BE IN ACCORDANCE WITH ACI 306.

5. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60.

6. UNLESS OTHERWISE NOTED, THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR **REINFORCEMENT:**

A) CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH - 3"

- B) CONCRETE EXPOSED TO EARTH OR WEATHER:
- #6 THROUGH #18 BARS 2"
- #5 BAR, W31 OR D31 WIRE & SMALLER- 1 1/2"

7. ALL REINFORCING MARKED CONTINUOUS (CONT.) ON THE PLANS AND DETAILS SHALL BE LAPPED 36 BAR DIAMETERS AT SPLICES UNLESS OTHERWISE INDICATED.

8. NO UNBALANCED BACKFILLING SHALL BE DONE AGAINST FOUNDATION WALLS UNLESS WALLS ARE SECURELY BRACED AGAINST OVERTURNING, EITHER BY TEMPORARY BRACING OR BY PERMANENT CONSTRUCTION.

9. PRIOR TO COMMENCING ANY FOUNDATION WORK, COORDINATE WORK WITH ANY EXISTING AND NEW UTILITIES. FOUNDATIONS SHALL BE STEPPED OR SLEEVED AS REQUIRED TO AVOID UTILITIES.

10. PROVIDE CONTROL JOINTS IN RETAINING WALLS AT APPROXIMATELY EQUAL INTERVALS NOT TO EXCEED 25 FEET NOR 3 TIMES THE WALL HEIGHT. PROVIDE EXPANSION JOINTS AT EVERY FOURTH CONTROL JOINT, UNLESS OTHERWISE INDICATED.

CONCRETE MASONRY NOTES

1. MASONRY CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE "SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530-13/ASCE 5-13/TMS 402-13)", PUBLISHED BY THE AMERICAN CONCRETE INSTITUTE, DETROIT, MICHIGAN.

2. THE MINIMUM DESIGN COMPRESSIVE STRENGTH OF THE MASONRY (fm) SHALL BE 2,000 PSI AT 28 DAYS AS DETEMINED BY THE UNIT STRENGTH METHOD USING TABLE 2 IN ACI 530. THE STRUCTURE IS SUPPORTED BY BEARING WALLS UNLESS NOTED OTHERWISE. ERECT MASONRY PRIOR TO CASTING CONCRETE COLUMNS WITHIN BEARING WALLS OR CASTING BEAMS AND SLABS SUPPORTED BY BEARING WALLS.

3. HOLLOW LOAD-BEARING MASONRY UNITS SHALL CONFORM TO ASTM C-90, GRADE N. BLOCK SHALL HAVE A NET AREA COMPRESSIVE STENGTH OF 2,800 PSI AND SHALL BE MANUFACTURED WITH NORMAL WEIGHT AGGREGATE.

4. THE USE OF MASONRY-CEMENT MORTAR IS STRICTLY PROHIBITED. MORTAR SHALL CONFORM TO ASTM C-270, TYPE S EXCEPT USE TYPE M MORTAR BELOW GRADE. ALL MORTAR SHALL MEET THE "PROPORTION SPECIFICATION" OF ASTM C-270 AND SHALL BE MADE WITH PORTLAND CEMENT/LIME (NON AIR-ENTRAINED). HEAD AND BED JOINTS SHALL BE 3/8" FOR THE THICKNESS OF THE FACE SHELL. WEBS ARE TO BE FULLY MORTARED IN ALL COURSES OF PIERS, COLUMNS AND PILASTERS; IN THE STARTING COURSE; AND WHERE AN ADJACENT CELL IS TO BE GROUTED. REMOVE MORTAR PROTRUSIONS EXTENDING 1/2" OR MORE INTO THE CELL.

5. FILL ALL BOND BEAMS AND REINFORCED CELLS SOLIDLY WITH FINE GROUT. GROUT SHALL CONFORM TO ASTM C-476 AND SHALL OBTAIN A MIN. 28 DAY COMPRESSIVE STRENGTH OF 2,500 PSI. AGGREGATE TO CONFORM TO ASTM C404 FOR FINE GROUT WITH A SLUMP OF 8" TO 10". GROUT ALL MASONRY CONTAINING REINFORCING, ALL CELLS OF 4 HOUR RATED WALLS, AND WHERE INDICATED ON THE DRAWINGS. ALLOW MORTAR TO CURE 24 HOURS PRIOR TO GROUTING. PROVIDE CLEANOUT OPENINGS AT THE BASE OF CELLS CONTAINING REINFORCING STEEL TO CLEAN THE CELL AND TIE THE VERTICAL BAR TO THE DOWEL. IN HIGH-LIFT GROUTING, USE 5'-0" MAXIMUM LIFTS, WITH 1/2 HOUR TO 1 HOUR BETWEEN LIFTS. VIBRATE EACH LIFT AND RECONSOLIDATE THE PREVIOUS LIFT.

6. REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ASTM A-615, GRADE 60, SHOP FABRICATE REINFORCING BARS WHICH ARE SHOWN TO BE HOOKED OR BENT. USE BAR SPACERS AT 10 FT. O.C. WHERE GROUT POUR HEIGHT EXCEEDS 10 FEET.

7. UNLESS OTHERWISE INDICATED, ALL WALLS SHALL BE LAID IN RUNNING BOND. SAWCUT UNITS WHICH ARE NOT IN MULTIPLES OF 8". UNITS SHALL BE AT LEAST 8" LONG. BOND CORNERS BY LAPPING 8" IN SUCCESSIVE VERTICAL COURSES.

8. PROVIDE VERTICAL REINFORCING BARS OF THE GIVEN SIZE AND SPACING AS INDICATED. PROVIDE BARS AT ALL WALL CORNERS. INTERSECTIONS AND OPENING EDGES. AT BOND/TIE BEAM CORNERS AND INTERSECTIONS, PLACE 1-#5x5'-6" TOP AND BOTTOM CORNER BAR WITH 32" LEGS EACH WAY, AT THE EXTERIOR FACE.

9. PROVIDE REBAR DOWELS FROM FOUNDATIONS TO MATCH VERTICAL REINFORCING SIZE AND SPACING. DOWELS SHALL HAVE STANDARD 90 DEGREE HOOKS AND LAP WITH THE FIRST LIFT OF REINFORCING.

10. PROVIDE HORIZONTAL BOND BEAMS WITH CONTINUOUS REINFORCING AS INDICATED. DISCONTINUE ALL HORIZONTAL REINFORCING AT CONTROL JOINTS EXCEPT FOR THE BOND BEAMS AT BEARING ELEVATIONS.

11. ALL VERTICAL WALL REINFORCING SHALL BE EXTENDED TO WITHIN 2" OF THE TOP OF ALL WALLS AND TERMINATE IN THE BOND BEAM WITH A STANDARD 90-DEGREE ACI HOOK.

13. REINFORCED MASONRY WALL CONSTRUCTION SHALL BE INSPECTED BY AN ENGINEER OR ARCHITECT IN ACCORDANCE WITH ACI 530.

14. THE MASONRY CONTRACTOR SHALL PROVIDE ALL REQUIRED TEMPORARY WALL BRACING DURING CONSTRUCTION (SEE "GENERAL STRUCTURAL NOTES").

16. PROVIDE LINTELS OR HEADERS AS SCHEDULED WITH MIN. 16" BEARING AT ALL EXTERIOR MASONRY OPENINGS. AT EXTERIOR OPENINGS EXTEND BARS 14" BEYOND THE FACE OF THE SUPPORT. FOR INTERIOR PARTITIONS EXTEND BARS 6" BEYOND THE FACE OF THE SUPPORT & PROVIDE 8" BEARING AT OPENINGS.

HEIGHT.

EXTERIOR COLD-FORMED METAL FRAMING

1. DESIGN, DETAIL AND ERECT EXTERIOR COLD-FORMED METAL FRAMING IN ACCORDANCE WITH THE GENERAL NOTES AND SPECIFICATIONS.

2. COLD-FORMED STEEL FRAMING DETAILS SHOWN ON CONTRACT DOCUMENTS REPRESENT THE MINIMUM DESIGN INTENT TO BE FOLLOWED. CONNECTIONS NOT DETAILED IN CONTRACT DOCUMENTS SHALL BE DESIGNED AND DETAILED BY THE DELEGATED ENGINEER ACCORDING. TO SPECIFICATIONS AND REQUIREMENTS HEREIN. THE MINIMUM DESIGN THICKNESS OF EXTERIOR COLD-FORMED METAL FRAMING SHALL BE 43 MILS. NO EXCEPTIONS WILL BE ACCEPTED.

3. SUBMIT COMPLETE SHOP DRAWINGS AND CALCULATIONS SHOWING METHOD OF FABRICATION, ERECTION PROCEDURES, ATTACHMENT OF THE SYSTEM TO THE BUILDING, JOINTS, CONNECTIONS AND FRAMING. CALCULATIONS AND SHOP DRAWINGS SHALL BE PREPARED, SIGNED AND SEALED BY THE DELEGATED ENGINEER LICENSED IN THE STATE OF FLORIDA.

4. USE GALVANIZED STEEL "C" STUDS, TRACKS, ANGLES AND STRAPS AS SHOWN ON DRAWINGS AND DETAILS HAVING A MINIMUM YIELD STRENGTH AS FOLLOWS:

43 MIL THICKNESS:

DOORS AND TO TRANSMIT THE FORCES TO THE PRIMARY STRUCTURAL FRAME.

6. ALL JAMBS, HEADERS, AND OTHER BUILT-UP MEMBERS SHALL BE CONSTRUCTED USING UNPUNCHED MATERIAL.

HAVE A PROTECTIVE COATING EQUIVALENT TO CADMIUM OR ZINC PLATING, ASTM B766.

10. LIMIT DEFLECTIONS OF STUDS BETWEEN SUPPORTS TO L/600 WHEN SUPPORTING MASONRY, L/360 WHEN SUPPORTING GYPSUM WALL BOARD OR PORTLAND CEMENT STUCCO AND L/240 FOR OTHER CONDITIONS.

MECHANICALLY FASTEN TO COLD-FORMED STEEL TRUSSES PER 1/S-6.0.

4. SUSPENDED CEILINGS, LIGHT FIXURES, DUCTS, AND OTHER PERMANENT SUSPENDED LOADS SHALL NOT BE SUPPORTED BY THE METAL DECK.

5. ALL ROOF DECKING SHALL BE GALVANIZED BY THE HOT-DIP PROCESS CONFORMING TO ASTM A924 CLASS G90. ALL DECK WELDS SHALL BE TOUCHED UP WITH GALVANIZING REPAIR PAINT FOR GALVANIZED DECKS.

6. SUBMIT DETAILED SHOP DRAWINGS PRIOR TO FABRICATION SHOWING LAYOUT, TYPES OF METAL DECK UNITS, CONNECTION DETAILS, ACCESSORIES AND OTHER RELATED ITEMS.

12. PROVIDE STANDARD 9 GAUGE HORIZONTAL JOINT REINFORCING AT 16" ON CENTER IN ALL WALLS. JOINT REINFORCING AND ANCHORS IN EXTERIOR WALLS SHALL CONFORM TO ASTM A153 CLASS B2, WITH A COATING THICKNESS OF 1.50 OZ/SF: CONFORM TO ASTM A641 IN INTERIOR WALLS. OVERLAP DISCONTINUOUS ENDS 6". USE PREFABRICATED CORNERS AND TEES. PROVIDE LADDER TYPE JOINT REINFORCING FOR ALL CONCRETE MASONRY. STOP ALL HORIZONTAL JOINT REINFORCING AT CONTROL JOINTS.

15. WHERE ANCHOR BOLTS, WEDGE ANCHORS OR ANCHORS SET IN EPOXY ARE SET IN A MASONRY WALL, FILL CELLS WITH GROUT FOR BOLTED COURSE, ONE COURSE ABOVE AND TWO COURSES BELOW. DO NOT SET MORE THAN ONE ANCHOR PER CELL.

17. WALL CONTROL JOINTS SHALL BE SPACED AT APPROXIMATELY EQUAL INTERVALS NOT TO EXCEED 25 FEET NOR 1.5 TIMES THE WALL

18. USE PRESSURE-TREATED WOOD FOR ALL WOOD IN CONTACT WITH MASONRY

54 MIL THICKNESS AND GREATER: Fy= 50 KSI MIN. Fy= 30 KSI MIN.

ALL TRACKS TO HAVE SAME DESIGN THICKNESS AS STUDS WITH MINIMUM 1 1/4 " LEGS.

5. DOOR AND WINDOW SILLS, HEADERS, AND JAMBS SHALL BE DESIGNED TO RESIST WIND FORCES ON TRIBUTARY WINDOWS AND

7. ALL JAMB STUDS SHALL BE FASTENED TOGETHER TO FORM A CONTINUOUS BUILT-UP SECTION.

8. SCREWS, WHERE REQUIRED, SHALL MEET THE MINIMUM REQUIREMENTS OF SAE J429 GRADE 5; AND IFI-105. SCREWS SHALL

9. FIELD CUTTING OF COLD-FORM METAL FRAMING SHALL BE BY SAW OR SHEAR. TORCH CUTTING IS NOT PERMITTED.

11. ADD WEB STIFFENERS AT CONCENTRATED LOADS AS REQUIRED BY DESIGN.

12. ALL WELDERS SHALL BE CERTIFIED BY AWS FOR THIN METAL SECTIONS.

NON-COMPOSITE METAL ROOF DECK

1. ALL METAL ROOF DECK SHALL BE MANUFACTURED AND ERECTED IN ACCORDANCE WITH THE "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS BY THE STEEL DECK INSTITUTE (SDI).

2. ALL METAL ROOF DECK SHALL BE 1 1/2" DEEP x 22 GAGE GALVANIZED TYPE B (WIDE RIB) DECK (MIN. Ip = 0.155 IN^4/FT AND Sp = 0.186 IN^3/FT) AS MANUFACTURED BY VULCRAFT OR APPROVED EQUAL.

3. ALL METAL ROOF DECK SHALL SPAN PERPENDICULAR TO SUPPORTS. WELD TO JOISTS AND STRUCTURAL STEEL PER 2/S-6.0.

7. ALL STEEL DECK SHALL BE CAPABLE OF SUPPORTING THE DESIGN LOADS PROVIDED IN THE CONTRACT DOCUMENTS. THE DECK SUPPLIER SHALL SUBMIT CALCULATIONS AND/OR LOAD TABLES WITH THE SHOP DRAWINGS. THESE CALCULATIONS AND/OR LOAD TABLES SHALL BE SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE. THE STRUCTURAL ENGINEEER OF RECORD'S REVIEW OF SUCH MATERIALS SHALL NOT RELIEVE THE DECK SUPPLIER AND ITS LICENSED ENGINEER OF THE SOLE RESPONSIBILITY FOR THE ACCURACY OF LOAD TABLES AND/OR CALCULATIONS SUBMITTED FOR REVIEW WITH THE SHOP DRAWINGS

WOOD FRAMING NOTES

1. ALL WOOD FRAMING MATERIAL SHALL BE SURFACED DRY AND USED AT 19% MAXIMUM MOISTURE CONTENT.

2. ALL WOOD FRAMING SHALL BE THE FOLLOWING: A. NO. 2 GRADE SOUTHERN YELLOW PINE (SYP)

3. ALL JOIST, TOP PLATE, AND MISCELLANEOUS FRAMING SHALL BE NO. 2 GRADE SOUTHERN YELLOW PINE U.N.O. 4. PROVIDE FULL-DEPTH BRIDGING AT MIDSPAN AND AT A MAXIMUM SPACING OF 8'-0" O.C.

5. ALL FRAMING EXPOSED TO THE WEATHER OR IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE-TREATED IN ACCORDANCE WITH THE AMERICAN WOOD PRESERVERS ASSOCIATION SPECIFICATIONS. WHERE POSSIBLE, ALL CUTS AND HOLES SHOULD BE COMPLETED BEFORE TREATMENT. CUTS AND HOLES DUE TO ON-SITE FABRICATION SHALL BE BRUSHED WITH 2 COATS OF COPPER NAPHTHENATE SOLUTION CONTAINING A MINIMUM OF 2% METALLIC COPPER IN SOLUTION (PER AWPA STD. M4).

6. THE CONTRACTOR SHALL CAREFULLY SELECT LUMBER TO BE USED IN LOADBEARING APPLICATIONS. THE LENGTH OF SPLIT ON THE WIDE FACE OF 2" NOMINAL LOADBEARING FRAMING SHALL BE LIMITED TO LESS THAN 1/2 OF THE WIDE FACE DIMENSION. THE LENGTH OF SPLIT ON THE WIDE FACE OF 3" (NOMINAL) AND THICKER LUMBER SHALL BE LIMITED TO 1/2 OF THE NARROW FACE DIMENSION.

7. NAILING SHALL BE IN ACCORDANCE WITH TABLE 2304.10.1 OF THE 2017 FLORIDA BUILDING CODE - BUILDING, 6TH EDITION.

8. PROVIDE DOUBLE JOISTS UNDER ALL PARTITIONS WHICH RUN PARALLEL WITH JOISTS AND UNDER ALL CONCENTRATED LOADS FROM FRAMING ABOVE.

9. PROVIDE HEADER BEAMS OF THE SAME SIZE AS JOISTS OR RAFTERS TO FRAME AROUND OPENINGS IN THE PLYWOOD DECK UNLESS OTHERWISE INDICATED.

10. STRUCTURAL STEEL PLATE CONNECTORS SHALL CONFORM TO ASTM A-36 SPECIFICATIONS AND SHALL BE 1/4" THICK UNLESS OTHERWISE INDICATED. BOLTS CONNECTING WOOD MEMBERS SHALL BE PER ASTM A-307 AND BE 3/4" DIAMETER UNLESS OTHERWISE INDICATED. PROVIDE WASHERS FOR ALL BOLT HEADS AND NUTS IN CONTACT WITH WOOD SURFACES.

11. BOLT HOLES SHALL BE CAREFULLY CENTERED AND DRILLED NOT MORE THAN 1/16" LARGER THAN THE BOLT DIAMETER. BOLTED CONNECTIONS SHALL BE SNUGGED TIGHT BUT NOT TO THE EXTENT OF CRUSHING WOOD UNDER WASHERS.

12. PREFABRICATED METAL JOIST HANGERS, HURRICANE CLIPS, HOLD-DOWN ANCHORS, AND OTHER ACCESSORIES SHALL BE AS MANUFACTURED BY "SIMPSON STRONG-TIE COMPANY", OR APPROVED EQUAL. INSTALL ALL ACCESSORIES PER THE MANUFACTURER'S REQUIREMENTS. ALL CONNECTORS AND FASTENERS SHALL HAVE SIMPSON ZMAX/HDG GALVANIZED COATING IN ACCORDANCE WITH ASTM A 153 OR SHALL BE A304 OR A316 STAINLESS STEEL.

13. HOLES AND NOTCHES DRILLED OR CUT INTO WOOD FRAMING SHALL NOT EXCEED THE REQUIREMENTS OF NDS 2015 OR 2017 FBC. BUILDING, 6TH EDITION.

14. ALL PLATES, ANCHORS, NAILS, BOLTS, NUTS, WASHERS, AND OTHER MISCELLANEOUS HARDWARE SHALL BE HOT DIP GALVANIZED OR SHALL BE A304 OR A316 STAINLESS STEEL.

PLYWOOD SHEATHING NOTES

SPECIFICATIONS.

2. ALL ROOF SHEATHING SHALL BE 3/4" (NOM.), TYPE C OR BETTER, STRUCTURAL I, EXTERIOR, WITH 24/16 SPAN RATING. SUITABLE EDGE SUPPORT SHALL BE PROVIDED BY USE OF PANEL CLIPS OR BLOCKING BETWEEN FRAMING. UNLESS OTHERWISE NOTED, CONNECT ROOF SHEATHING WITH 10d RING-SHANK NAILS AT 4" O.C. AT SUPPORTED PANEL EDGES AND 6" O.C. AT INTERMEDIATE SUPPORTS. PROVIDE 2X BLOCKING AT PANEL EDGES IF INDICATED ON THE ROOF FRAMING PLAN.

3. INSTALL ALL PLYWOOD ROOF SHEATHING WITH THE LONG DIMENSION OF THE PANEL ACROSS SUPPORTS AND WITH PANEL CONTINUOUS OVER TWO OR MORE SPANS. STAGGER PANEL END JOINTS. ALLOW 1/8" SPACING AT PANEL ENDS AND EDGES UNLESS OTHERWISE RECOMMENDED BY THE SHEATHING MANUFACTURER.

EXISTING CONSTRUCTION NOTES

1. BEFORE PROCEEDING WITH ANY WORK WITHIN THE EXISTING FACILITY, THE CONTRACTOR SHALL BECOME FAMILIAR WITH EXISTING STRUCTURAL AND OTHER CONDITIONS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE ALL NECESSARY SAFEGUARDS TO MAINTAIN ALL PARTS OF THE EXISTING WORK IN A SAFE CONDITION DURING THE PROCESS OF DEMOLITION AND CONSTRUCTION AND TO PROTECT FROM DAMAGING THOSE PORTIONS OF THE EXISTING WORK WHICH ARE TO REMAIN.

2. THE CONTRACTOR SHALL FIELD VERIFY THE DIMENSIONS, ELEVATIONS, ETC. NECESSARY FOR THE PROPER CONSTRUCTION AND ALIGNMENT OF THE NEW PORTIONS OF THE WORK TO THE EXISTING WORK. THE CONTRACTOR SHALL MAKE ALL MEASUREMENTS NECESSARY FOR FABRICATION AND ERECTION OF STRUCTURAL MEMBERS. ANY DISCREPENACY SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ENGINEER.

JOB SITE SAFETY

1. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR JOB SITE SAFETY AND FOR CONFORMANCE WITH THE HEALTH AND SAFETY PROVISIONS REQUIRED BY THE FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION AND ANY AND ALL REGULATORY AGENCIES. THE ENGINEER HAS NO AUTHORITY TO EXERCISE CONTROL OVER THE WORK OR ANY HEALTH AND SAFETY PRECAUTIONS OF ANY CONSTRUCTION CONTRACTOR, SUBCONTRACTOR OR THEIR EMPLOYEE.

ERECTION, BRACING, AND FORMWORK

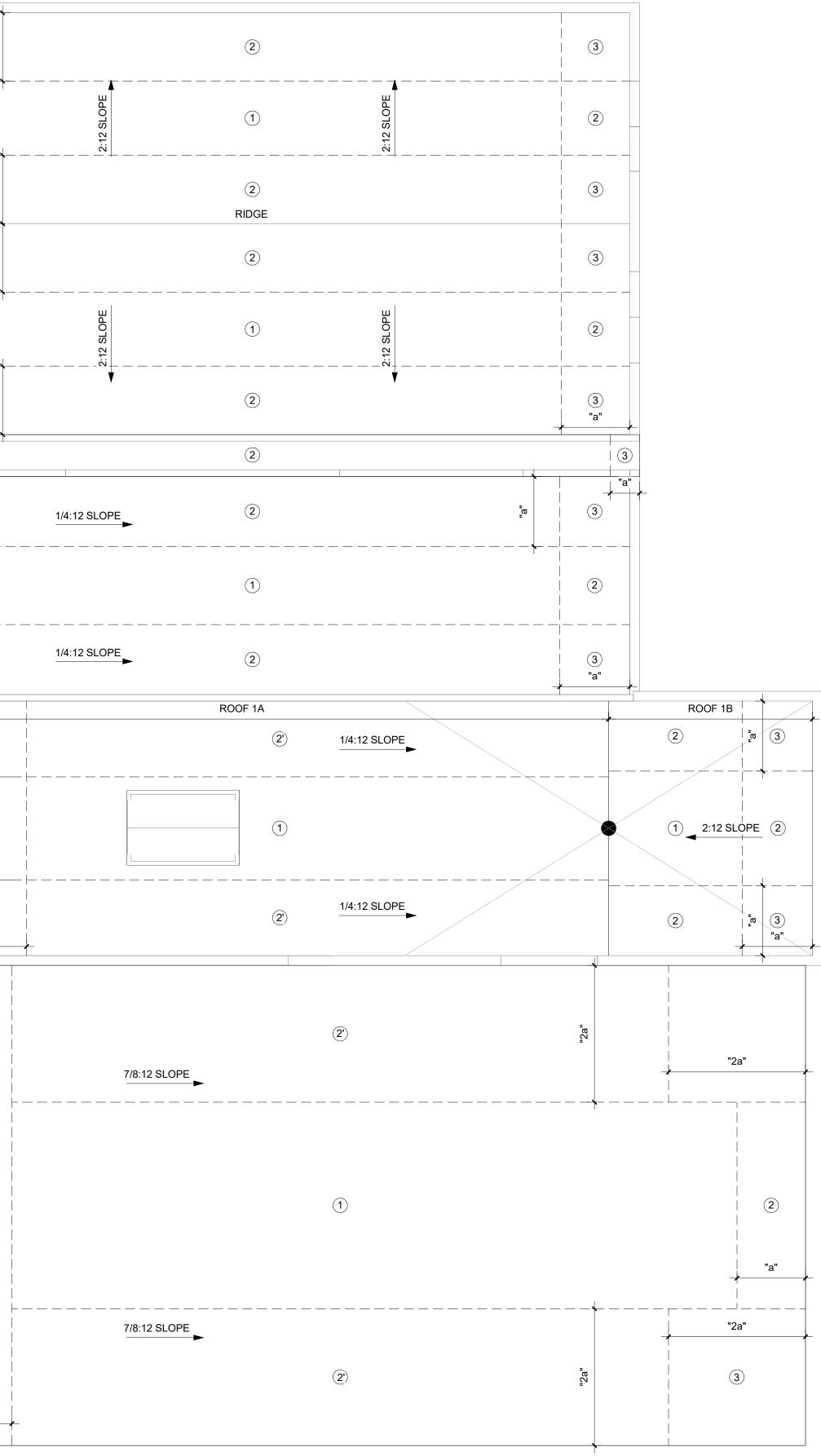
1. THE DESIGN, ADEQUACY, AND SAFETY OF ERECTIO, BRACING, FORMWORK, SHORING, OR OTHER TEMPORARY MEANS OF SUPPORT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

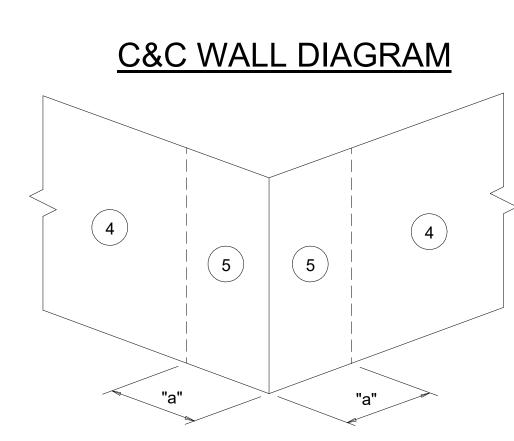
1. ALL PLYWOOD CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE AMERICAN PLYWOOD ASSOCIATION (APA)

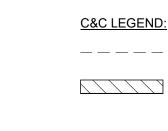
4. ALL NAILING SHALL BE CAREFULLY DRIVEN AND NOT OVERDRIVEN.



3 2 (3) (2) 3 "a" 3 | / / (3) 1/4:12 SLOPE RATIVE ROOF 2 2 2_ _ _ _ _ _ _ _ _ 1/4:12 SLOPE 3 "a" -/---ROOF 1A (3') (3') "2a" 7/8:12 SLOPE _ _ _ _ _ _ 3 7/8:12 SLOPE "2a"







C&C LEGEND:

— — — — — WIND LOAD SEPARATION OVERHANG



COMPONENTS AND CLADDING WIND PRESSURES ULTIMATE WIND PRESSURES AT LEWIS BUILDING

Building Length (L) = 152.0 ft	
Least Width (B) = 100 ft	
Type of Roof = Monoslope	
Roof Pitch = 7/8 : 12	

ROOF ULTIMATE WIND PRESSURES (LOW END) EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF)

ZONE	AREA, SF	PRESSURE	SUCTION
1	10	+16.0	-30.5
	20	+16.0	-30.5
	50	+16.0	-30.5
	100	+16.0	-30.5
2	10	+16.0	-35.3
	20	+16.0	-34.5
	50	+16.0	-33.6
	100	+16.0	-32.9
3	10	+16.0	-47.2
	20	+16.0	-42.9
	50	+16.0	-37.2
	100	+16.0	-32.9

a = 7' - 0"

ROOF ULTIMATE WIND PRESSURES (HIGH END)

	EFFECTIVE WIND	WIND PRESSURE AND SUCTION (PS			
ZONE	AREA, SF	PRESSURE	SUCTION		
2'	10		-42.4		
	20		-41.7		
	50		-40.7		
	100		-40.0		
3'	10		-66.2		
	20		-59.0		
	50		-49.6		
	100		-42.4		
a = 7' - 0"					

WALL ULTIMATE WIND PRESSURES EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF)

	-				
ZONE	AREA, SF	PRESSURE	SUCTION		
4	10	+25.7	-27.9		
	20	+24.6	-26.7		
	50	+23.1	-25.2		
	100	+21.9	-24.1		
	500	+19.3	-21.4		
5	10	+25.7	-34.3		
	20	+24.6	-32.0		
	50	+23.1	-29.0		
	100	+21.9	-26.7		
	500	+19.3	-21.4		
$r_{r} = 7'$ 0" Note: GCni reduced by 10% due to roof angle < 10°					

a = 7' - 0" Note: GCpi reduced by 10% due to roof angle \leq 10°.

COMPONENTS AND CLADDING WIND PRESSURES ULTIMATE WIND PRESSURES AT ADMINISTRATIVE BUILDING (ROOF 1A)

Building Length (L) = 152.0 ft Least Width (B) = 100 ft Type of Roof = Monoslope Roof Pitch = 2 : 12

Mean Roof Height (h) = 17 ft Kh(case 1) = 0.7 Base Pressure (qh) = 23.8 psf GCpi = +/-0.55

ROOF ULTIMATE WIND PRESSURES (LOW END)

EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF) AREA SE PRESSURE SUCTION

ZONE	AREA, SF	PRESSURE	SUCTION
1	10	+20.2	-39.3
	20	+19.5	-39.3
	50	+18.6	-39.3
	100	+17.9	-39.3
2	10	+20.2	-44.1
	20	+19.5	-43.4
	50	+18.6	-42.4
	100	+17.9	-41.7
3	10	+20.2	-56.0
	20	+19.5	-51.7
	50	+18.6	-46.0
	100	+17.9	-41.7

a = 7' - 9"

ROOF ULTIMATE WIND PRESSURES (HIGH END)

	EFFECTIVE WIND	WIND PRESSURE A	ND SUCTION (PSF)
ZONE	AREA, SF	PRESSURE	SUCTION
2'	10		-51.2
	20		-50.5
	50		-49.5
	100		-48.8
3'	10		-75.0
	20		-67.9
	50		-58.4
	100		-51.2

PARAPET WIND PRESSURES			
	EFFECTIVE WIND	WIND PRESSURE AN	ID SUCTION (PSF)
ZONE	AREA, SF	PRESSURE	SUCTION
INT.	10	+52.4	-45.0
	20	+50.5	-42.7
	50	+48.1	-39.7
	100	+46.2	-37.4
	500	+43.6	-32.2
CORNER	10	+64.3	-51.5
	20	+58.9	-48.0
	50	+51.7	-43.5
	100	+46.2	-40.1
	500	+43.6	-32.2
2 – 7' 0"			

a = 7' - 9"

WALL ULTIMATE WIND PRESSURES FFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF)

	EFFECTIVE WIND	ND WIND PRESSURE AND SUCTION	
ZONE	AREA, SF	PRESSURE	SUCTION
4	10	+34.5	-36.7
	20	+33.4	-35.5
	50	+31.9	-34.0
	100	+30.8	-32.9
	500	+28.1	-30.3
5	10	+34.5	-43.1
	20	+33.4	-40.8
	50	+31.9	-37.8
	100	+30.8	-35.5
	500	+28.1	-30.3
a = 7' - 9"	Note: GCpi reduced by 10% due to roof angle \leq 10		angle ≤ 10°.

CORNER

INT.

a = 7' - 2"

a = 7' - 2"

Building Leng Least Width (Type of Roof = Roof Pitch = 1/4 : 12

ZONE 1 2

a = 7' - 2"

ZONE 1 & 2 O.H.

3 O.H.

a = 7' - 2"

COMPONENTS AND CLADDING WIND PRESSURES

ULTIMATE WIND PRESSURES AT ADMINISTRATIVE BUILDING (ROOF 1B)

gth (L) = 152.0 ft	Mean Roof Height (h) = 17 ft
(B) = 100 ft	Kh(case 1) = 0.7
f = Monoslope	Base Pressure (qh) = 23.8 pst
1/4 : 12	GCpi = +/-0.55

ROOF ULTIMATE WIND PRESSURES

EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF) PRESSURE SUCTION AREA SE

AREA, SF	PRESSURE	SUCTION
10	+20.2	-36.9
20	+19.5	-36.2
50	+18.6	-35.3
100	+17.9	-34.5
10	+20.2	-56.0
20	+19.5	-51.0
50	+18.6	-51.5
100	+17.9	-39.3
10	+20.2	-79.8
20	+19.5	-67.6
50	+18.6	-51.5
100	+17.9	-39.3

ROOF OVERHANG ULTIMATE WIND PRESSURES EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF)

	AREA, SF	PRESSURE	SUCTION
	10		-40.5
	20		-39.8
	50		-38.8
	100		-38.1
	10		-66.7
	20		-52.4
	50		-33.4
	100		-19.1

Overhang pressures in the table above assume an internal pressure coefficient (GCpi) of 0.0. Overhand soffit pressure equals adjacent wall pressure (which included internal pressure of 11.3.

PARAPET WIND PRESSURES EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF)

ZONE AREA. SF PRESSURE SUCTION

_		TRESSORE	00011014
	10	+64.3	-45.0
	20	+58.2	-42.7
	50	+50.0	-39.7
	100	+43.9	-37.4
	500	+41.2	-32.2
	10	+88.1	-51.5
	20	+74.8	-48.0
	50	+57.2	-43.5
	100	+43.9	-40.1
	500	+41.2	-32.2

WALL ULTIMATE WIND PRESSURES

EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF) SUCTION AREA, SF PRESSURE 10 +34.5 -36.7

20	+33.4	-35.5	
50	+31.9	-34.0	
100	+30.8	-32.9	
500	+28.1	-30.3	
10	+34.5	-43.1	
20	+33.4	-40.8	
50	+31.9	-37.8	
100	+30.8	-35.5	
500	+28.1	-30.3	
Note: GCpi reduced by 10% due to roof angle ≤ 10°.			

COMPONENTS AND CLADDING WIND PRESSURES

ULTIMATE WIND PRESSURES AT ADMINISTRATIVE BUILDING (ROOF 2)

Building Length (L) = 152.0 ft Least Width (B) = 100 ft Type of Roof = Monoslope

Roof Pitch = 1/4 : 12

Mean Roof Height (h) = 17 ft Kh(case 1) = 0.7

Base Pressure (qh) = 23.8 psf GCpi = +/-0.55

ROOF ULTIMATE WIND PRESSURES EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF)

		WIND FILLSOURE P	
ZONE	AREA, SF	PRESSURE	SUCTION
1	10	+20.2	-36.9
	20	+19.5	-36.2
	50	+18.6	-35.3
	100	+17.9	-34.5
2	10	+20.2	-56.0
	20	+19.5	-51.0
	50	+18.6	-51.5
	100	+17.9	-39.3
3	10	+20.2	-79.8
	20	+19.5	-67.6
	50	+18.6	-51.5
	100	+17.9	-39.3
a = 7' - 2"			

ROOF OVERHANG ULTIMATE WIND PRESSURES			
EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF ZONE AREA, SF PRESSURE SUCTION			
1 & 2 O.H.	10		-40.5
	20		-39.8
	50		-38.8
	100		-38.1
3 O.H.	10		-66.7
	20		-52.4
	50		-33.4
	100		-19.1
a = 7' - 2" Overhang pressures in the table above assume an internal			

pressure coefficient (GCpi) of 0.0. Overhand soffit pressure equals adjacent wall pressure (which included internal pressure of 11.3.

PARAPET WIND PRESSURES EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF) ZONE AREA, SF PRESSURE SUCTION

INT.	10	+64.3	-45.0
	20	+58.2	-42.7
	50	+50.0	-39.7
	100	+43.9	-37.4
	500	+41.2	-32.2
CORNER	10	+88.1	-51.5
	20	+74.8	-48.0
	50	+57.2	-43.5
	100	+43.9	-40.1
	500	+41.2	-32.2
a = 7' - 2"			

WALL ULTIMATE WIND PRESSURES			
	EFFECTIVE WIND	WIND PRESSURE AN	ID SUCTION (PSF)
ZONE	AREA, SF	PRESSURE	SUCTION
4	10	+34.5	-36.7
	20	+33.4	-35.5
	50	+31.9	-34.0
	100	+30.8	-32.9
	500	+28.1	-30.3
5	10	+34.5	-43.1
	20	+33.4	-40.8
	50	+31.9	-37.8
	100	+30.8	-35.5
	500	+28.1	-30.3
a = 7' - 2"	Note: GCpi re	educed by 10% due to ro	of angle ≤ 10°.

COMPONENTS AND CLADDING WIND PRESSURES ULTIMATE WIND PRESSURES AT ADMINISTRATIVE BUILDING (ROOF 3)

Building Length (L) = 79.3 ft Least Width (B) = 4.3 ft Type of Roof = Monoslope

Roof Pitch = 1/8 : 12

ROOF ULTIMATE WIND PRESSURES EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF) ARFA SF

ZONE	AREA, SF	PRESSURE	SUCTION
1	10	+20.2	-36.9
	20	+19.5	-36.2
	50	+18.6	-35.3
	100	+17.9	-34.5
2	10	+20.2	-56.0
	20	+19.5	-51.0
	50	+18.6	-44.3
	100	+17.9	-39.3
3	10	+20.2	-79.8
	20	+19.5	-67.6
	50	+18.6	-51.5
	100	+17.9	-39.3

a = 3' - 0" ft	

ROOF OVERHANG ULTIMATE WIND PRESSURES			
ZONE	EFFECTIVE WIND AREA, SF	WIND PRESSURE	AND SUCTION (PSF) SUCTION
1 & 2 O.H.	10		-40.5
	20		-39.8
	50		-38.8
	100		-38.1
3 O.H.	10		-66.7
	20		-52.4
	50		-33.4
	100		-19.1
a = 3' - 0" ft	0" ft Overhang pressures in the table above assume an internal pressure coefficient (GCpi) of 0.0. Overhand soffit pressure		

equals adjacent wall pressure (which included internal pressure of 11.7.

PARAPET WIND PRESSURES				
	EFFECTIVE WIND	WIND PRESSURE A	AND SUCTION (PSF)	
ZONE	AREA, SF	PRESSURE	SUCTION	
NT.	10	+64.3	-45.0	
	20		40.7	

INT.	10	+64.3	-45.0	
	20	+58.2	-42.7	
	50	+50.0	-39.7	
	100	+43.9	-37.4	
	500	+41.2	-32.2	
CORNER	10	+88.1	-51.5	
	20	+74.8	-48.0	
	50	+57.2	-43.5	
	100	+43.9	-40.1	
	500	+41.2	-32.2	
a = 3' - 0"				

WALL ULTIMATE WIND PRESSURES EFFECTIVE WIND WIND PRESSURE

	EFFECTIVE WIND	WIND PRESSURE	
ZONE	AREA, SF	PRESSURE	
4	10	+34.5	
	20	+33.4	
	50	+31.9	
	100	+30.8	
	500	+28.1	
5	10	+34.5	
	20	+33.4	
	50	+31.9	
	100	+30.8	
	500	+28.1	
a = 3' - 0" ft	Note: GCpi re	educed by 10% due to	

Mean Roof Height (h) = 20 ft Kh(case 1) = 0.7 Base Pressure (qh) = 23.8 psf GCpi = +/-0.55

PRESSURE SUCTION

AND SUCTION (PSF)	
SUCTION	
-36.7	
-35.5	
-35.5	
-34.0	
-32.9	
-30.3	
-43.1	
-40.8	
-37.8	
-35.5	
-30.3	

due to roof angle $\leq 10^{\circ}$.

COMPONENTS AND CLADDING WIND PRESSURES ULTIMATE WIND PRESSURES AT ADMINISTRATIVE BUILDING (ROOF 4)

Building Length (L) = 152.0 ft Least Width (B) = 100 ft Type of Roof = GABLE Roof Pitch = 2:12

a = 7' - 0"

a = 7' - 0"

Mean Roof Height (h) = 17 ft Kh(case 1) = 0.7 Base Pressure (qh) = 23.8 psf GCpi = +/-0.55

ROOF ULTIMATE WIND PRESSURES

EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF) PRESSURE SUCTION AREA. SF

ZONE	AREA, SF	PRESSURE	SUCTION
1	10	+25.0	-34.5
	20	+23.6	-33.8
	50	+21.7	-32.9
	100	+20.2	-32.2
2	10	+25.0	-53.6
	20	+23.6	-50.0
	50	+21.7	-45.3
	100	+20.2	-41.7
3	10	+25.0	-75.0
	20	+23.6	-70.7
	50	+21.7	-65.0
	100	+20.2	-60.7

ROOF OVERHANG ULTIMATE WIND PRESSURES

	EFFECTIVE WIND	WIND PRESSURE	AND SUCTION (PSF)
ZONE	AREA, SF	PRESSURE	SUCTION
2 O.H.	10		-52.4
	20		-52.4
	50		-52.4
	100		-52.4
3 O.H.	10		-88.1
	20		-79.5
	50		-68.2
	100		-59.6

Overhang pressures in the table above assume an internal pressure coefficient (GCpi) of 0.0. Overhand soffit pressure equals adjacent wall pressure (which included internal pressure of 11.2.

PARAPET WIND PRESSURES

	EFFECTIVE WIND	WIND PRESSURE AN	ND SUCTION (PSF)
ZON	AREA, SF	PRESSURE	SUCTION
INT.	10	+61.9	-45.0
	20	+57.2	-42.7
	50	+51.0	-39.7
	100	+46.2	-37.4
	500	+43.6	-32.2
CORNER	10	+83.4	-51.5
	20	+77.9	-48.0
	50	+70.7	-43.5
	100	+65.3	-40.1
	500	+62.6	-32.2
a = 7' - 0"			

WALL UI TIMATE WIND PRESSURES

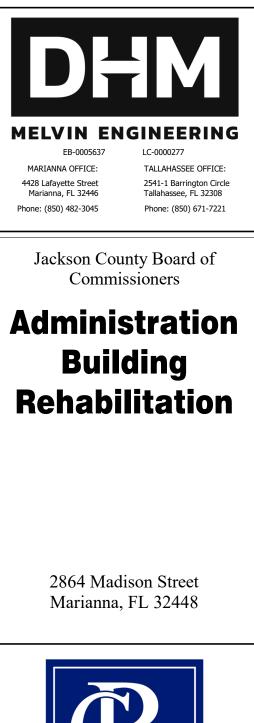
	WALL ULTIMATE WIND PRESSURES		
	EFFECTIVE WIND WIND PRESSURE AND SUCTION (PSF)		
ZONE	AREA, SF	PRESSURE	SUCTION
4	10	+34.5	-36.7
	20	+33.4	-35.5
	50	+31.9	-34.0
	100	+30.8	-32.9
	500	+28.1	-30.3
5	10	+34.5	-43.1
	20	+33.4	-40.8
	50	+31.9	-37.8
	100	+30.8	-35.5

+28.1

-30.3

a = 7' - 0" Note: GCpi reduced by 10% due to roof angle \leq 10°.

500





Clemons, Rutherford. & Associates, Inc.

Architects Planners Interior Designers Construction Managers

2027 Thomasville Road Tallahassee, Florida 32308

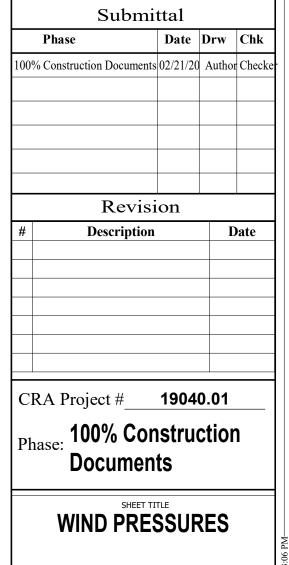
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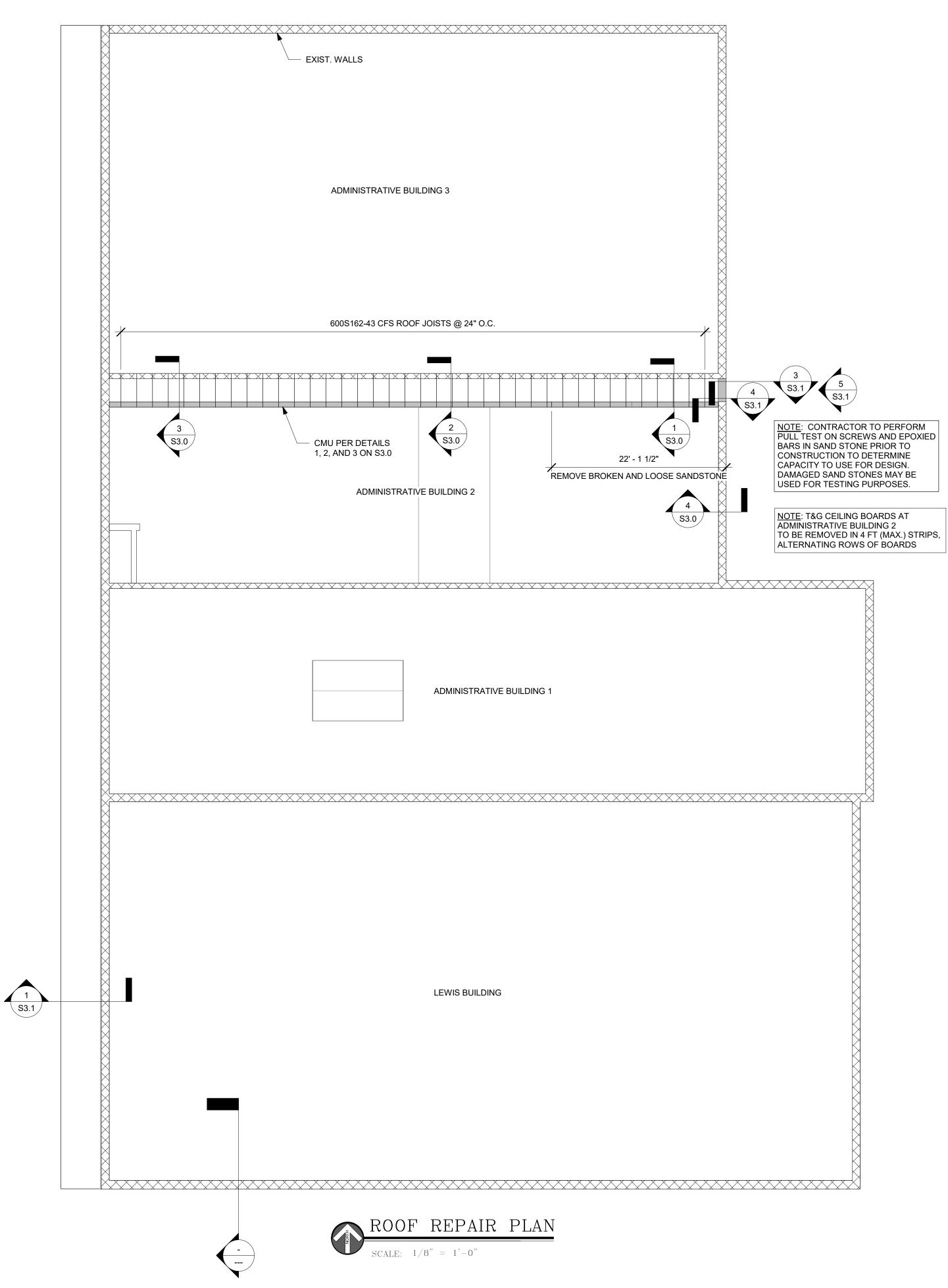
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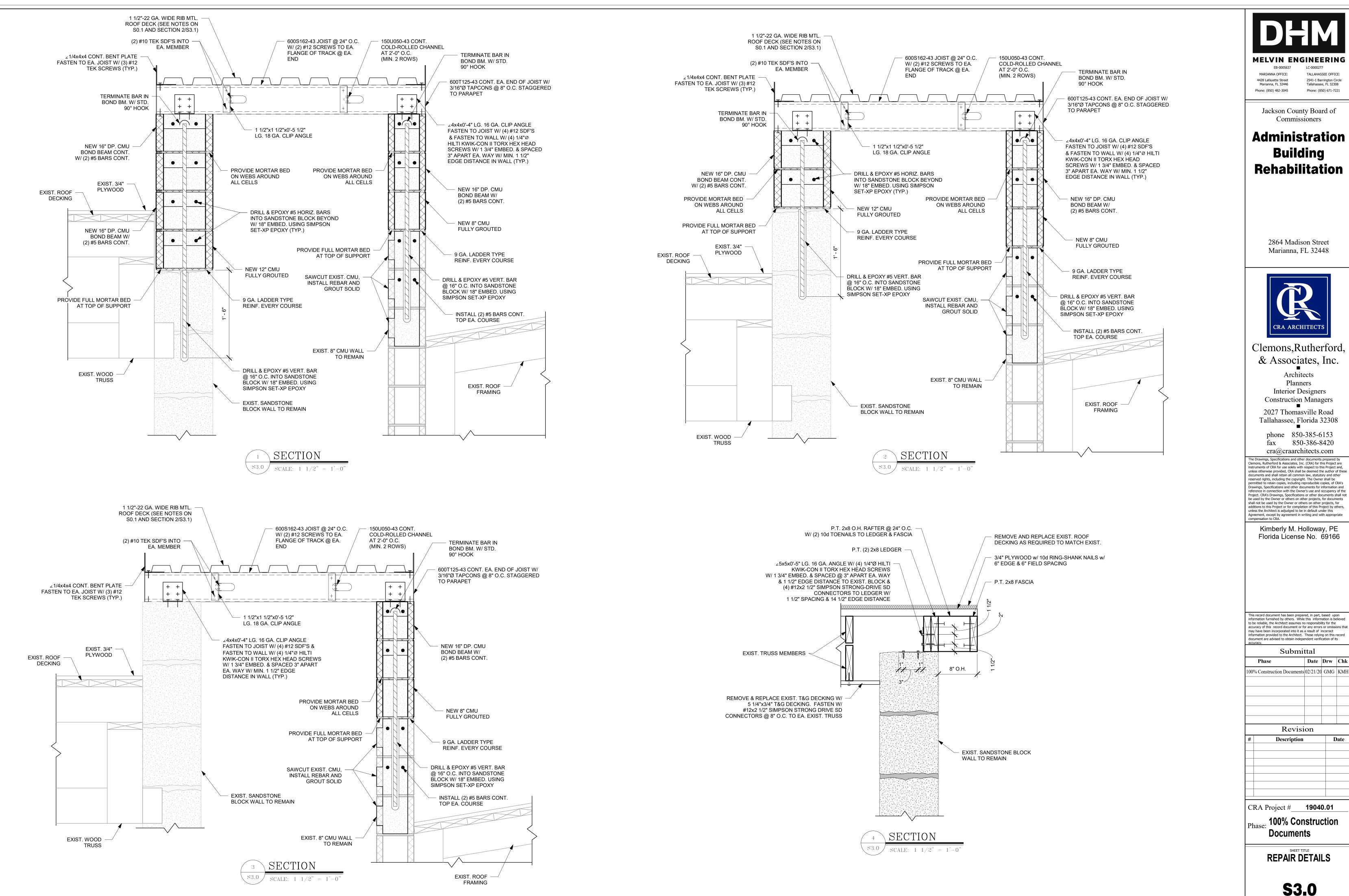
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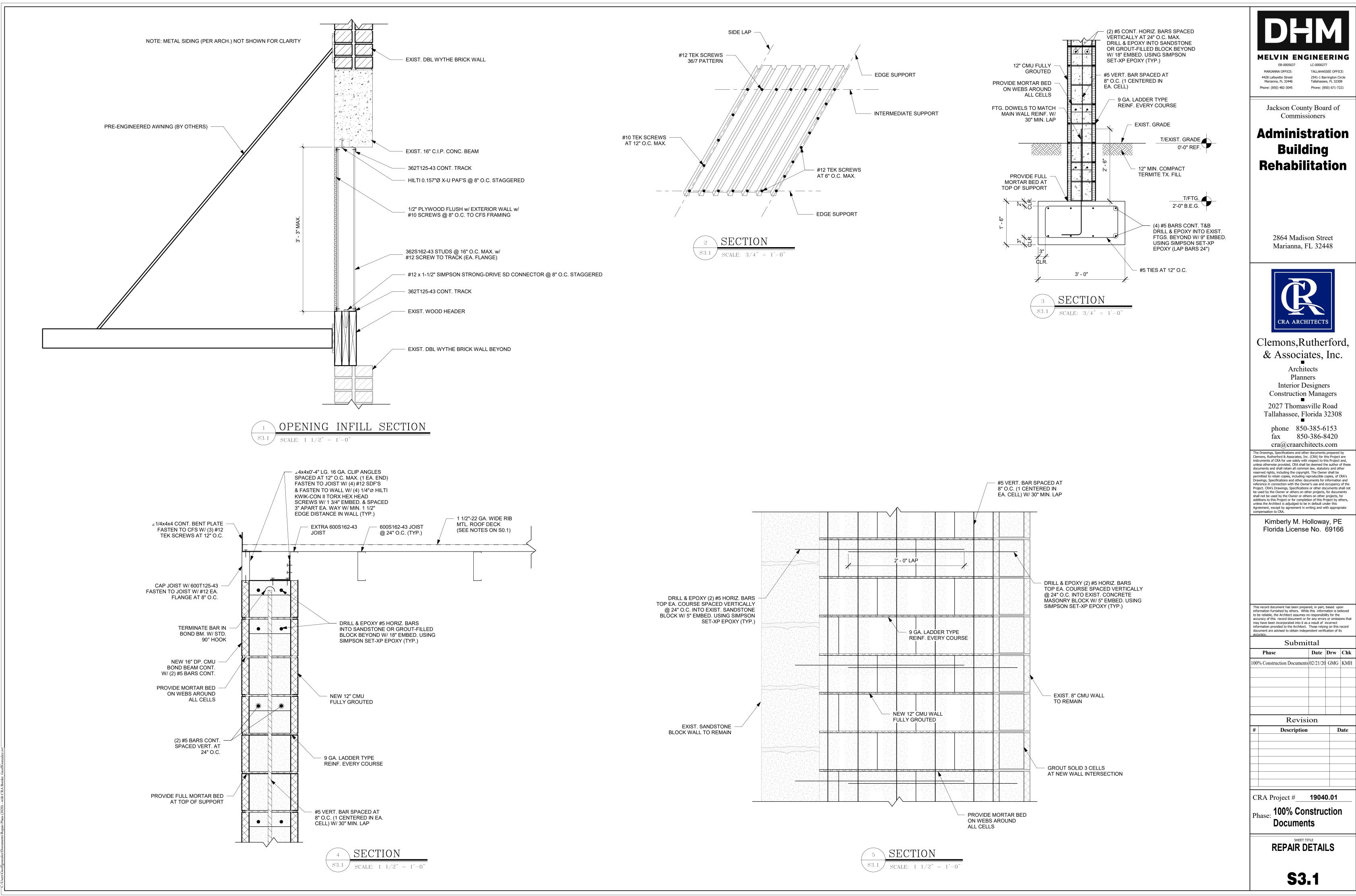




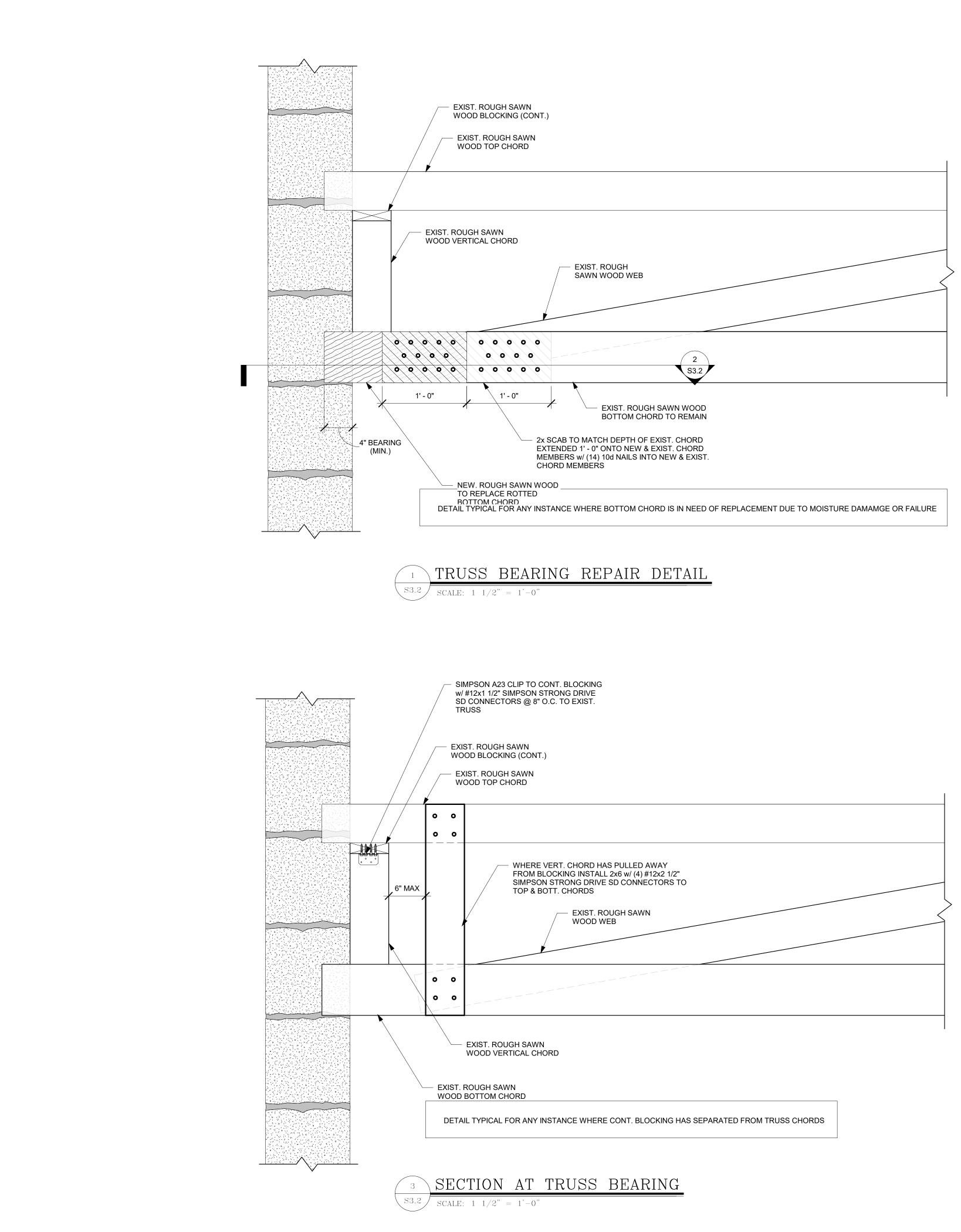








IDE RIB CK N S0.1)				
	DRILL & EPOXY (2) #5 HORIZ. BARS – TOP EA. COURSE SPACED VERTICALLY @ 24" O.C. INTO EXIST. SANDSTONE BLOCK W/ 5" EMBED. USING SIMPSON SET-XP EPOXY (TYP.)		2' 	- 0" LAP
				9 GA. LADDER TYPE REINF. EVERY COU
				EW 12" CMU WALL



EXIST. ROUGH SAWN

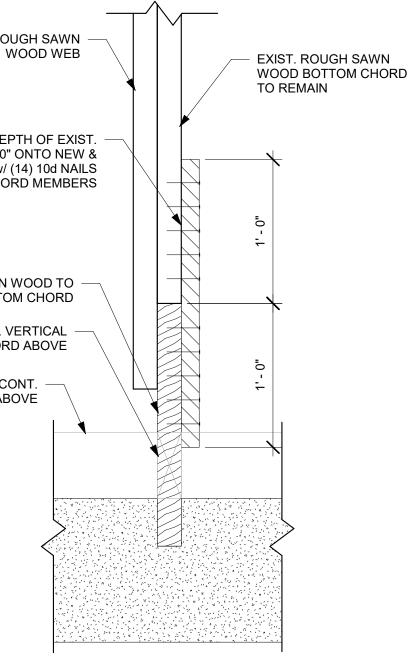
2x SCAB TO MATCH DEPTH OF EXIST. CHORD EXTENDED 1'-0" ONTO NEW & EXIST. CHORD MEMBERS w/ (14) 10d NAILS INTO NEW & EXIST. CHORD MEMBERS

NEW. ROUGH SAWN WOOD TO -REPLACE ROTTED BOTTOM CHORD

> EXIST. VERTICAL CHORD ABOVE

EXIST. CONT. **BLOCKING ABOVE**





² TRUSS BEARING REPAIR DETAIL (TOP VIEW)

